AN ENGINEERING GEOLOGICAL INVESTIGATION OF THE URBAN LAND AUTHORITY'S PROPERTY, MELTON

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### ABSTRACT

An engineering geological investigation was carried out on land at Melton to determine the viability of subdivision and house construction. The site has two serious limitations - deep expansive soils and active soil subsidence - which will require expensive engineered solutions. A description of the geology and pedology is detailed and the potential development hazards, together with some possible engineering applications are discussed.

### KEYWORDS

Engineering Geology, Site investigation, Gilgai, Soils

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#### INTRODUCTION

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A request from the Urban Land Authority was received on 16 October 1984 for engineering geology advice on the area known as the "sink-hole plain" at Bimbadeen Estate, Melton. Following subsequent negotiations, the service of the Office of Minerals and Energy (OME) was engaged to carry out an investigation in order to assess the geological constraints on the proposed urban development of the site. Of specific interest are the geological parameters which would affect the subdivision density, domestic housing foundations, sewage effluent disposal and road making. Investigative works commenced in Febuary 1985.

### 2 SCOPE OF THIS REPORT

This report specifically addresses the engineering geological conditions which are of concern to the urban development of the site. It covers the physical details of the site; relevant site history and previous work in the area; investigative work carried out; the nature, causes and engineering significance of potential hazards; and suggested solutions.

Recommendations are made in respect to the site geological conditions only and do not take into account any other factors (such as the preservation of the environment) which may affect development decisions.

### 3 SITE DETAILS 3.1 Location

The site is situated approximately 40km W.N.W. of Melbourne in the N.W. corner of the Melton development area (Fig. 1). The area of the site is approximately 50Ha and it is bounded by Harkness Road to the west, farmland to the north and a proposed regional cemetery to the south and east.

### 3.2 Vegetation

The vegetation on the site comprises mostly eucalypts - grey box and some yellow gum - which vary in size (5 to 50cm diam.; 2 to 20 m high). The larger trees have been removed in the past as indicated by a number of stumps and some coppice regrowth.

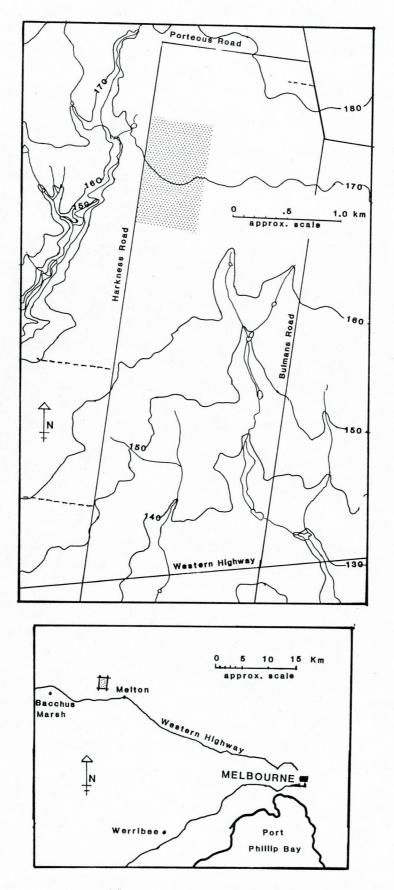
Understorey vegetation (mostly boxthorn bushes) is scattered among the trees and along the fences.

#### 3.3 Land use

The area is currently owned by the ULA and until recently was used for grazing.

Discussion with the adjoining property owner, Mr T Minn, revealed that the treed portion of the site had been used for grazing within his memory (over 60 years) and had probably never been ploughed.

An historic air-photo (11/11/1931) confirms this observation and shows the site as being almost unchanged from its present condition.



# Fig.1 LOCATION SKETCH PLAN

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#### 3.4 Topography

The most striking features of the site are the surface undulations. These vary from broad depressions up to 1.5 metres deep to smaller "sinkholes" only one metre wide but one metre deep. Some gentle mounding is obvious, although depressions far out-number the mounds.

The mechanism causing the subsidence is still apparently active since "sinkhole" development has been observed over the past three years. Mr Minn has observed that although the depressions have always been there, they appear to be getting bigger and more numerous.

The undulations are largely confined to the treed area, particularly the recent subsidence, although the immediately surrounding areas contain some broad depressions.

The causes and significance of these depressions are discussed in sections 7.2, 7.3 and 8.2

### 4 PREVIOUS WORK AND LITERATURE

4.1 Investigations by the Geological Survey

The earliest known investigations in the area were carried out in 1894 when a bore was sunk just to the north of the site as part of a regional investigation for minerals. The stratigraphic sequence was recorded as:

3 ft 2 in surface clay, 100 ft 5 in basaltic rocks, 14 ft 6 in yellow and light clays, 3 ft 11 in gravelly clay and gravel, 4 ft 6 in gravel or fine washdirt; bottomed on sandstone at 126 ft 6 in.

No samples of this bore are recorded at the core store. In 1958 Kenley mapped the site as being covered by Djerriwarrh High Level Gravels, Pliocene(?) to middle Tertiary in age. In 1974 VandenBerg mapped the gravels covering the site as being Darley Gravels of Pliestocene age.

In 1983 engineering geological mapping of the Melton proposed. development area was commenced by this author. As part of this project shallow bores were drilled along Harkness, Porteous and Bulmans Roads. The soil core recovered from these bores was logged and some samples were tested. Some standpipes were later installed to monitor any shallow groundwater, however none has been recorded.

Following a request by the Health Commission in August 1984 an investigation was carried out on a large tract of land owned by the ULA, to determine its suitability for the establishment of a regional cemetery. The area investigated included the "sinkhole plain" where drilling and backhoe excavations were carried out. The report concluded that a large portion of the area was suitable for cemetery development, however the undulating area was not.

### 4.2 Other Investigations

4.2.1 The geology and physiography of the region has been previously described by Kitson (1901) who described the area along the lower Werribee valley; Fenner (1918) who wrote a long and detailed account of the physiography of the Werribee River; Condon (1948 & 1950) who carried out a detailed survey of the lower Werribee River; and Forbes (1948) who studied the erosion of the Melton Reservoir catchment .

4.2.2 The Soil Conservation Authority (SCA) reported on "Erosion and its control during urban and drainage development of the Melton area" in 1978. This report dubbed the site the "sinkhole plain" and showed it as a separate land unit. A brief description of the site is included in the report and it is suggested that further investigation be carried out before the land-use is changed. The SCA report was prepared for the Dandenong Valley Authority (DVA) who completed a "Melton drainage study" of the development area in 1980.

4.2.3 The Melton-Sunbury interim co-ordinating committee published six volumes of planning strategies, produced by consultants in the period 1976-77. The physical structure concept plan of the site is presented in Volume 3 (Clarke Gazzard Planners - 1976); The site is recommended as an area for "infill and redevelopment residential/commercial", although it is also ranked as an area of natural environmental significance and regarded as having recreation potential.

### 5 ADDITIONAL SITE INVESTIGATION WORK 5.1 Drilling

Two bores were drilled to depths of 15.5 m and 15.9 m respectively using the OME's Gemco 210B rotrary drilling rig. The soil profile was sampled using hollow augers and thin-walled sampling tubes. Diamond drilling was carried out in the underlying rock and NQ rock core was retreived for examination.

### 5.2 Geophysics

Geophysical exploration was conducted in order to establish the nature of the soil-rock interface. Two refraction seismic survey lines were run using a 24 channel seismograph, a 3 m geophone spacing and gelignite as an energy source. The results of this survey are presented in Appendix III.

### 5.3 Site Examination

Site visits were conducted with collegues Messrs A M Cooney and M K Cecil (10/5/1984), and Mr J L Neilson (10/10/84) of the OME; and Mr J Maher, Scientific Officer, from the Department of Agriculture (21/3/85). The on-site discussions were fruitful in the search for a subsidence mechanism.

### 6 GEOLOGY

6.1 Regional Geology

VandenBerg's 1974 Sunbury sheet (1:63,360) shows the regional geology of the area as being Upper Ordovician sediments overlain by Tertiary (Pliocene) Bullengarook Gravels, Tertiary (Pliocene) Newer Volcanics and Quaternary (Pleistocene) Darley Gravel. More recent work has re-positioned the Djerriwarrh Fault to the west of the site and re-established an ancient eruption point or maar to the north. Roberts (1984) does not accept the name Darley Gravels for the post-Newer Volcanic material, prefering to refer to them as simply alluvial outwash.

The regional geology is summarized in figure 2.

6.2 Site Geology

No rock outcrop occurs on site, hence the geology is interpreted from borehole information, geophysical exploration, surface mapping and regional trends.

### 6.2.1 Upper Ordovician.

The basement rock of the area is regarded as the sediments of the Upper Ordovician age. These outcrop along Djerriwarrh Creek to the west of the site and were encountered in the boreholes drilled on the site. In outcrop, they typically consist of closely and steeply folded interbedded shales, sandstones and greywacke. In the core retreived from the bores this horizon is represented by fine to medium grained, yellow-brown sandstone with some quartz veining.

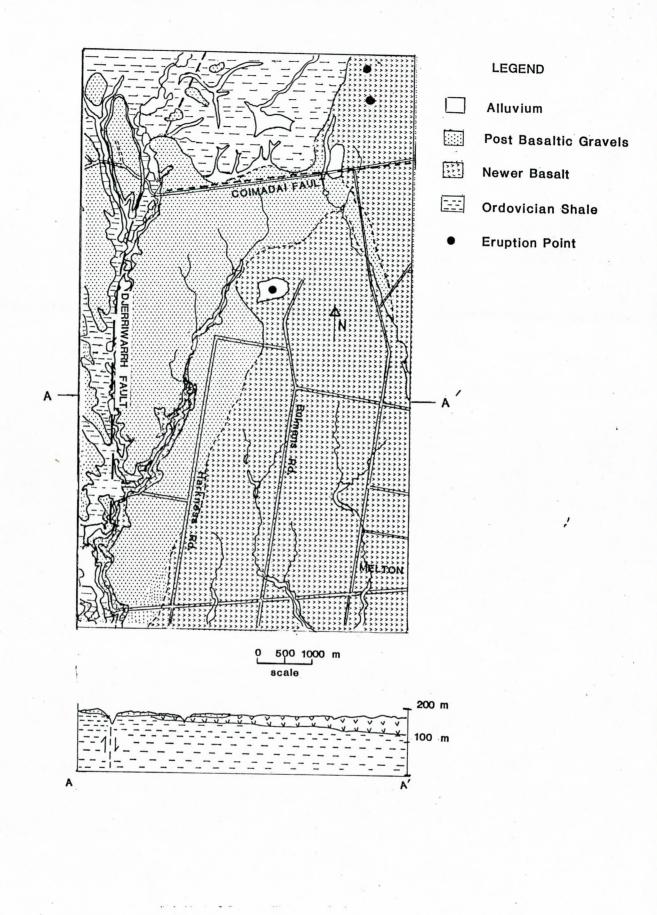




Fig 2

### 6.2.2 Tertiary (Pliocene) Gravels

Unconformably resting on the Upper Ordovician sediments are the Pliocene gravels, sands, silts and clays of the Bullengarook Gravel formation. These are exposed along Djerriwarrh Creek as unconsolidated silty sands and gravels with occassional ferruginous horizons. In the boreholes, core recovery was poor through this section, with the material retreived being mostly clayey sands and gravels. Some carborate-cemented sands were recovered from the lower portion of the section in both bores.

6.2.3 Tertiary (Pliocene) Newer Volcanics.

Disconformably overlying the Bullengarook Gravel are the ancient lava flows which make up the Newer Volcanics. These are exposed on the bank of Djerriwarrh Creek and shown in the boreholes to be olivine basalts, ranging from fresh to completely weathered and dense to vesicular. The thickness of the basalt increases regionally to the east, as shown by the cross-section in figure 2. It appears that only one basalt flow, approximately 4 metres thick, is represented in the bores on site.

The depth of weathering, represented by the soil thickness, is unusually deep for the Newer Volcanics of the Melton area and may reflect that the parent material was of pyroclastic origin. The source for such material could be the eruption point to the north of the site, which has the same dimensions and gravity signature as a maar (L. Thomas pers. comm.).

Scoria is present in bores along Porteous and Harkness Roads to the north and east of the site, although at a depth of only two metres or so. This adds weight to the theory of the eruption point being explosive, although whether it provided the parent material for the deep plastic clays remains academic.

# 6.2.4 Quaternary (Pleistocene) gravels

Overlying the basaltic clays of the weathered Newer Volcanics is a thin variable layer of silty sands and gravels of Pliestocene age. These have been variously described in previous literature as shoreline deposits (Kitson 1901), alluvial outwash (Fenner 1918), and lake deposits (Condon 1948). VandenBerg (1974) and Roberts (1984) regard these post-basaltic gravels to be alluvial outwash derived from the Ordovician rocks of the Coimadai Fault scarp, 4 kilometres to the north of the site.

The thickness of the deposit varies from a centimetre or so, to half a metre in places around the site. The boundary between the gravels and the basaltic clay is indistinct, since intermixing of the units has occurred as a result of both natural pedological processes and land-use practices.

6.2.5 The site geology is illustrated in figure 3.

Located in pocket at rear of report.

6.3 Groundwater

Standpipes installed in bores Djerriwarrh 100 and 105, along Harkness Road, were monitored over a period of 12 months without recording any groundwater in the soil horizons.

In both recent bores (Djerriwarrh 192 and 193) water levels were recorded as detailed in Table 2, below.

BORE NAME AND NUMBER	DATES AND 17/4/85	LEVELS 23/4/85	2/5/85	18/6/85
Djerriwarrh 192	14.4 m	14.4 m	14.5 m	14.4 m
Djerriwarrh 193	12.6 m	12.6 m	12.5 m	12.5 m

Table Water table levels recorded on site.

The recorded levels are unusually high for the Melton area, although not inconsistent with the geology. Water bores drilled in the region generally encounter the water table either in or just below the basalt. The water quality and quantity are variable and it is only used for stock purposes.

#### 7 PEDOLOGY

7.1 Nature of the Soil 7.1.1 Profile

The soil type varies in the upper profile, but generally consists of gravelly, sandy and/or clayey silts overlying silty and/or sandy clays. Lower in the profile the soil is predominately highly plastic clay with some sandy horizons and rare gravel layers.

### 7.1.2 Types of Material

The sands and gravels are typically quartzose, sub-angular to sub-rounded, and well graded. The size of material is generally less than 10mm, although occasional pieces as large as 100mm were noted on the surface.

The clay fraction is largely composed of highly expansive material. X-Ray Diffraction (XRD) analyses of clay material (identical in appearance) sampled from a bore adjacent to the site (Djerriwarrh 93) showed a predominance of montmorillinite in the clay fraction of the sample.

The expansiveness of the clay fraction is displayed by extensive surface cracking in areas where the sands and silts do not overlie the clayey soils (Plate 1). In the profile (as seen from backhoe excavations and core recovered from drilling) the presence of numerous fissures, some filled with sand and others with slickenslided surfaces (Plate 2), testify to the depth of cracking. In all bores drilled on site the fissuring extends the full depth of the soil profile (over 9 metres), with small tree rootlets noted following down the fissure planes.

### 7.2 Nature of the Surface Undulations 7.2.1 Field Observations

From observations at the site, the shape of the depressions may be divided into three categories:

### 7.2.1.1 Saucer Shaped Depressions

These depressions are generally 2-5 m wide and up to 10 m long. They have no sharp edges and are from 1-1.5 m deep. Very often they appear to have developed along a line and nearly always have a single crack (15 cm wide and several metres deep) along the bottom. The soil in the depression is darker than the surrounding soil since it is rich in organic matter. (Plates 3 & 4).

### 7.2.1.2 Saucer Shaped Depression with a Sharp Edge

These depressions are similar to those described above, except that they have a sharp drop at the edge on one side of the depression. The drop is typically 0.5-1.5 m or so, and located above the crack in the bottom of the depression. The depression grows larger by erosion of the sharp edge into the centre crack. (Plates 5 & 6).

#### 7.2.1.3 "Sinkholes"

The "sinkholes" are essentially circular depressions occurring on either flat ground or in the bottom of the saucer shaped depressions. They are generally less than 2 m wide and sharp edged. They have been observed to unpredictably develop within a day or so, particularly during or immediately following rain. The depressions are reasonably deep (1.5 m or so) and often have large cracks in the soil at their base. (Plates 7 & 8).

Over successive years, the "sinkholes" develop (by erosion) into the depressions described in 7.2.1.2 above.

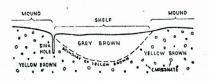
### 7.2.1.4 Mounds

The mounds are gentle rises which generally form the rim if the depressions described in sections 7.2.1.1 & 7.2.1.2. The surface soil appears unchanged from the flat areas. Mounds are not as numerous as the depressions.

### 7.2.2 Comparisons to Other Areas 7.2.2.1 Comparisons with "Gilgai" Areas

"Gilgai", taken from an Aboriginal word for a small water hole, is used to describe small-scale undulations on the surface of the land which are particularly widespread in Australia. They were first described in soil literature by Prescott (1931) and subsequently by various other writers (notably Hallsworth, Robertson and Gibbons-1955; Hallsworth and Beckmann-1969; and Knight-1972). The term broadly encompasses a number of different forms and magnitudes of undulations which have the common feature of a more expansive subsoil than surface soil in the depressed area.

Gilgai forms have been classified into six categories (Hallsworth et al. 1955 & 1969) :-i normal or round gilgai, melon-hole gilgai, lattice gilgai, linear or wvy gilgai, tank gilgai and stony gilgai. Of these the normal and melon-hole forms show some resemblance to the observed features at Melton. These are diagramatically represented in Figure 4 and compare well to the depressions described in sections 7.2.1.1 and 7.2.1.2.



Normal gligal of the Riverina Taken near Leeton, N.S.W.



Melon hole gligal from Gundarimba, North Coast, (from Hallsworth & Beckmann- 1969)` 14

#### Fig. 4 Gligal Terminology



Plate 1 Cracking of clay soils. (photographed 11/2/'85; scale is 50mm diameter)



Plate 2 Slickensliding of fissure in soil core.

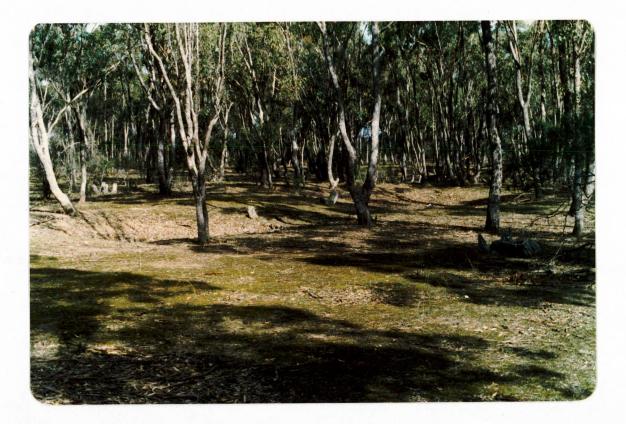


Plate 3 Broad saucer shaped depressions





Plate 5 Sharp edge of depression



# Plate 6 Depression with sharp edge eroding into centre crack



Plate 7 'Sinkhole' (photographed in road reserve, Spring 1983)



### 7.2.2.2 Comparisons with Other "Sinkholes"

The term sinkhole in the geological meaning is normally restricted to a circular depression in a karst area. In recent soil literature however, the term has been used to describe various depressions occurring in the soil surface, which are not normally related to karst features.

In the terminology developed to describe gilgai morphology 'sinkhole' is used in reference to roughly circular holes which occur on the shelf or at the bottom of the depression (Figure 4).

Hallsworth and Beckmann (1969) and Stapledon (1970) described some depressions/sinkholes which occur over large areas of the lower Murray Basin. A sketch section through one of these (Figure 5)in the Tilmy Flat area of South Australia resembles the "sinkholes" described in 7.2.1.3. (Plate 9). Although the resemblance is striking, the geology is quite different, with the clay at Tilmy Flat being underlain by sand.

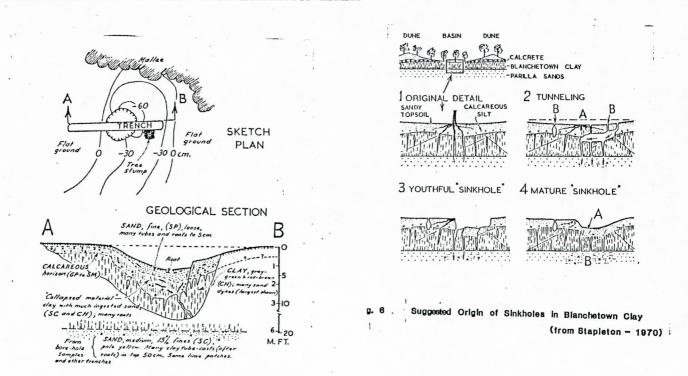


Fig. 5 Tillmy Flat Area, Sketch Plan and Trench Section through a Large Sinkhole. (from Stapleton - 1970)



Plate 9 Section through 'skinhole' (excavation pit-Djerriwarrh 176 - 10/5/'84)

### 7.3 Suggested Origin of the Undulations

From field observation it is apparent that the development of the broad saucer-shaped features begin with a "sinkhole". The material from the edge of the hole is washed into the crack at the base of the hole and eventually the depression becomes a large saucer-shaped feature. The addition of the surface material to the subsoil (ie. the sandy material washed down the crack) begins the development of mounds and depressions as described by the literature on gilgai development.

The initial development of the "sinkhole" is more problematic. For the features described at Tilmy Flat, the suggested origin (Figure 6) relies on the clay being underlain by sand. At this site however, the clay continues to the basalt bedrock, with no obvious highly permeable layers to channel dispersed clay away. This becomes apparent when the "sinkholes" hold water at the end of wet periods (Plate 10).

During on-site discussions with Mr J Maher (Scientific Officer, Dept. of Agriculture) a possible mechanism was formulated which may account for the origin of the "sinkholes". It was noted that the development of the features was largely confined areas where:

- the sand/silt/gravel covered the clay
- the clay was substantially thicker
- there were trees

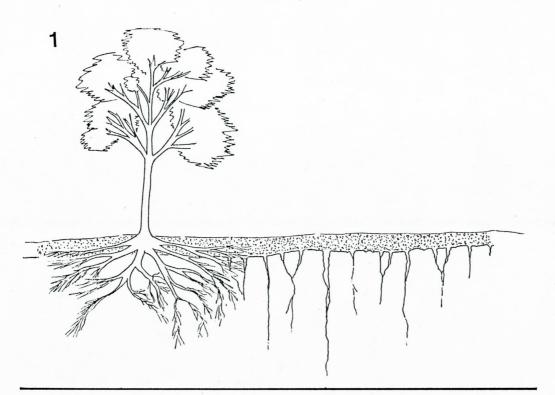
Keeping in mind the expansiveness of the clay, the following suggested origin seems plausible:-

The trees and associated vegetation dessicate the clay substantially, especially in the drier seasons, and form deep cracks and many fissures in the clay. The cracks do not extend to the surface however, since the sand/silt/gravels of the surface are non-plastic and do not shrink on drying. Some of the surface material finds its way into the cracks in the underlying clay, especially during the rainy periods when the relatively high permeability of the surface quickly percolates the rainwater to the near impervious clay layer, where cracks and fissures provide the only drains. The substantial depth of clay provides the space for a reasonable volume of material to be accommodated in the fissures. The erosion of the base of the sand/silt/gravel into the clay fissures creates a void which is not expressed on the surface because the cemented nature of the material. Eventually the void collapses dramatically to create the "sinkhole". The suggested mechanism is shown in Figure 7.

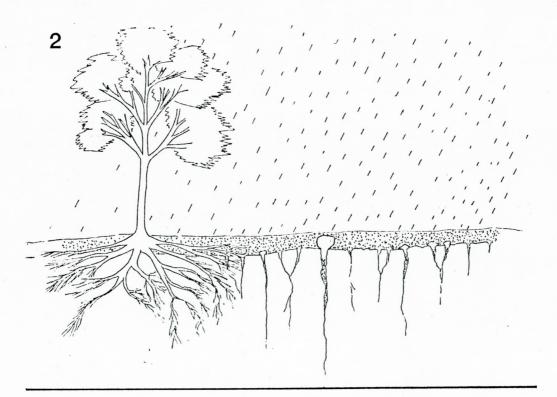
One problem however, remains unsolved. That is the observation by Mr Minn that the "sinkholes" still occur on his property which is outside of the treed area. Although his adjacent paddock is ploughed each year, the subsidence still occurs, although not with the same frequency or extent as in the treed area. The phenomenon of recurring gilgai has been reported by Hallsworth, Robertson and Gibbons (1955) as typical of gilgai areas.



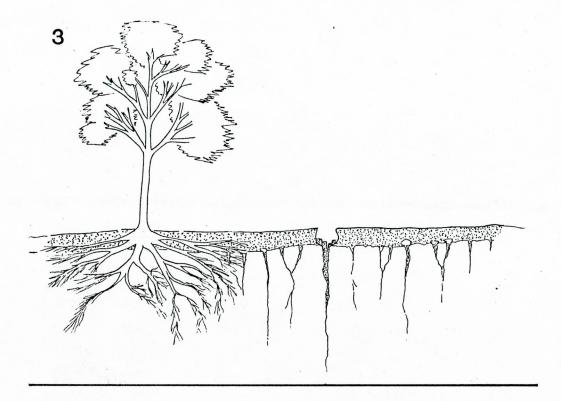
Plate 10 Water filled depressions (Spring 1983)



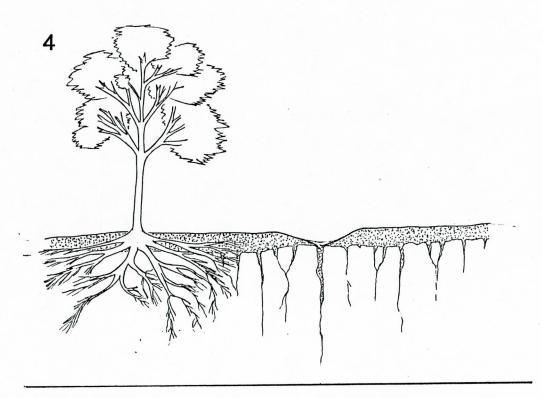
The vegetatation dessicates the soil, causing the highly expansive clay subsoil to shrink and crack deeply, whereas the non-expansive sandy topsoil remains unaffected.



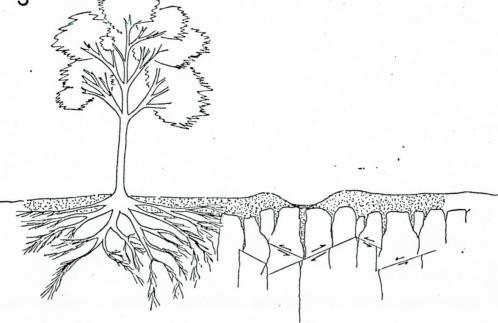
The silty, sandy topsoil falls into the shrinkage cracks in the clay, particularly during rainy periods, when the rainwater easily percolates through the sand and down the cracks in the clay. A void is created in the lightly cemented silty sands above the deeper cracks.



The topsoil eventually collapses into the void creating the "sinkholes" which appear dramatically.



The surrounding soll is eroded into the "sinkhole", smoothing the sides to form a saucer-shaped depression.



The subsequent swelling of the subsoil creates stresses which are relieved by fissuring and slickensliding of the clay, which buckles the surface on the edge of the depression. SUGGESTED ORIGIN OF SUBSIDENCE FEATURES OF MELTON Fig 7

#### 8 GEOLOGICAL HAZARDS FOR DEVELOPMENT 8.1 Expansive Clay

The presence of thick expansive clay soil poses a hazard to the development of the site. Buildings, roads and sewerage systems are susceptible to serious damage from the season movements of the soil.

No preferred locations which would avoid the clays can be chosen for building construction. The only viable solution would be to have building foundations engineer-designed to tolerate considerable season movements.

#### 8.2 Subsidence

The most obvious problem for development of the site is the active subsidence. Roads, housing, and sewerage systems would be seriously damaged by sinkhole development. The unpredictability of the sinkholes makes the siting of any structures hazardous. It is likely however, that areas of the site could be found where the potential for subsidence would be lower.

# 9 POSSIBLE ENGINEERING SOLUTIONS

9.1 Housing

The presence of deep basaltic clay classifies the site as "unstable" for strip and stump footings and "intermediate" for slabs or footing slabs according to the Victoria Building Regulations (VBR's). The regulations set out the minimum dimensions for either type of footing, which are not condidered adequate for this site. The depth of highly expansive clay and the potential for sinkhole development create unusual foundation conditions which require special engineering design for footings.

Buildings founded on expansive soil need careful attention paid to building design and maintenance, in order to mitigate or control structural damage. Properly engineered foundations, segmented interior design, flexible connections to utility lines, and carefully designed lot drainage and landscaping are required for satisfactory building performance.

Selection of building sites in areas where the soil is thinnest and removal of all trees surrounding buildings would lessen the risk of subsidence occurring, although not entirely rule it out. Placement of the footings on the rock (by designing pier and beam footings) would ensure that the building would not subside, even though the soil may. Chen (1975) warns that pier and beam design does not always work in expansive soils, since the swelling and shrinking can produce considerable lateral and frictional forces on the piers.

An alternative solution would be to replace the foundation soils with non-swelling granular soils. Chen (1975) suggests at least 1.5 m under the footings and 3 m beyond the building line (Appendix 3). Soil replacement will lessen the chances of building distress considerably since it would overcome the effects of the expansive soils and cushion the effect of any subsidence. The possibility of subsidence occuring still remains, although the effects would be less dramatic on the surface due to the compensatory movement of the granular soil.

A new approach to site classification has been recommended in the Draft Australian Standard - 'Residential Slabs and Footings' - (August 1985), which would classify this site as both a class E (Extremely reactive) site and a class P (Problem) site. The design of footing systems for these classifications are outlined in sections 4 and 5 of the draft standard, which should be consulted before final footing design is decided upon.

### 9.2 Roadmaking

The subdivision of the land into more than one lot may necessitate the construction of permanent roads. This is also problematic because of the nature of the soils of the site.

The Road Construction Authority have extensively tested similar soils for the construction of the Western Freeway Melton By-pass and opted for lime stabilization as a suitable soil treatment. The addition of lime (approx. 4%) to the subgrade material greatly improves the roadmaking properties and is recommended for all roads constructed on the site.

The subsidence problem still remains, however. Harkness Road is periodically maintained by the Shire of Melton and any "sinkholes" in the road or road shoulders are filled with crushed rock. The continuous traffic makes the development of the holes less dramatic and if the trees were removed from the roadside reserve the problem would be lessened.

Any paved or compacted surfaces should be provided with adequate drainage to lessen the potential for heaving of the road centreline. Similarly, to lessen the impact of seasonal variations in moisture content, the road edges and shoulders should be well drained and maintained. On no account should trees or shrubs be planted along the road reserves, as they would locally dessicate the soil and almost certainly create "sinkholes".

9.3 Sewage Disposal

Because the site is isolated from the Melton City by a proposed regional cemetery, is was not intended by the ULA to service the site with reticulated sewerage.

Alternative sewage disposal would be limited to above ground methods (e.g. composting, chemical or incinerating toilets, "grey water" irrigation, etc.), since the permeability of the clay is too low to provide adequate effluent absorption. Even in areas where septic system absorption lines could be located in the gravel/sand/silt layer, the localised addition of moisture to the underlying expansive clay would cause excessive swell and distress in the sewerage system. Similarly, sand filters or other in-ground disposal would be ultimately unsatisfactory.

### 10 RECOMMENDATIONS 10.1 Sub-division Density

The following factors need be considered when planning the sub-division density:-

a construction of building foundations will be costly;
 b complete removal of trees surrounding buildings is
 essential; and

c roads need to be well constructed and maintained.

10.2 Buildings

Buildings should be sited on shallower soils where feasable, and constructed using either pier-and-beam footings or an engineer designed slab over an adequate layer of non-plastic soil.

Trees must be kept well away from buildings and good site drainage is essential.

10.3 Roads

Roads should be constructed by lime stabilisation of the subgrade and well maintained.

10.4 Sewage disposal

Sewage effluent should not be disposed of in-ground if at all possible. Septic tank systems and sand filters would not be acceptable.

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	RTMENT OF MINERALS AND ENERGY		GE	OLOGICAL LOG OF DRILL HOLE	HOLE NO. DJERRIWARRH	93
LOCA R.L. G	TION Harkness Rd. Melton. V.L. BROUND 166.4 m				AMG REF. 5829801.88 282651.13	
DEPTH R.L.	SOIL DESCRIPTION type, plasticity, particle characteristics, colour, accessory material	S. U.S.C. SYMBOL	IC LOG	SOIL additional observations, defects, test results		
(m)	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components	R. WEATHERING	GRAPHIC	ROCK MASS DEFECTS TYPE, INCLINATION, THICKNESS, ROUGHNESS, COATING/INFILL		,
	SILTY CLAY,					
0.1	moist, blocky, dark brown.					
	dark brown.					
0.2						
	SILTY CLAY					
0.3	SILTY CLAY, moist, very shiff,					
	brown.					
0.4						
0.5						
	SILTY CLAY, Dandy monist, Very Shiff, brown-grey.					
0.6	mait 100					
	moust, very shift,					
0.7	brown-grey.		44			
'	0 0					
5.8						
0.9						
1.0						
1.1						
1.2						
	SILTY CLAY					
1.3	monist very aliPl					
	indui, vory stift,					
1.4	morried yellow-brown					
	and grey.					
1.5	SILTY CLAY, moist, very shiff, mottled yellow-brown and grey. sandy in parts.					
	0				•	
1.6						
5						
ŀ7						•
'						
1-8						
			<u> </u>			
1.9						
'						
				· · · · · · · · · · · · · · · · · · ·		
DRILL T	YPE Genio 2108 LOGGED	- J. P.	Fisher Duhlha	NOTES		
CORE 8	ARREL HOLOW Flight DATE		5.6.83			
COMME		R	. Wilson	-		
COMPLE	ETED 6.6.83 DRAWN					
VERTIC	AL SCALE 1:10	s	HEET (	OF 3 DRA	WING NO. M 131/1	

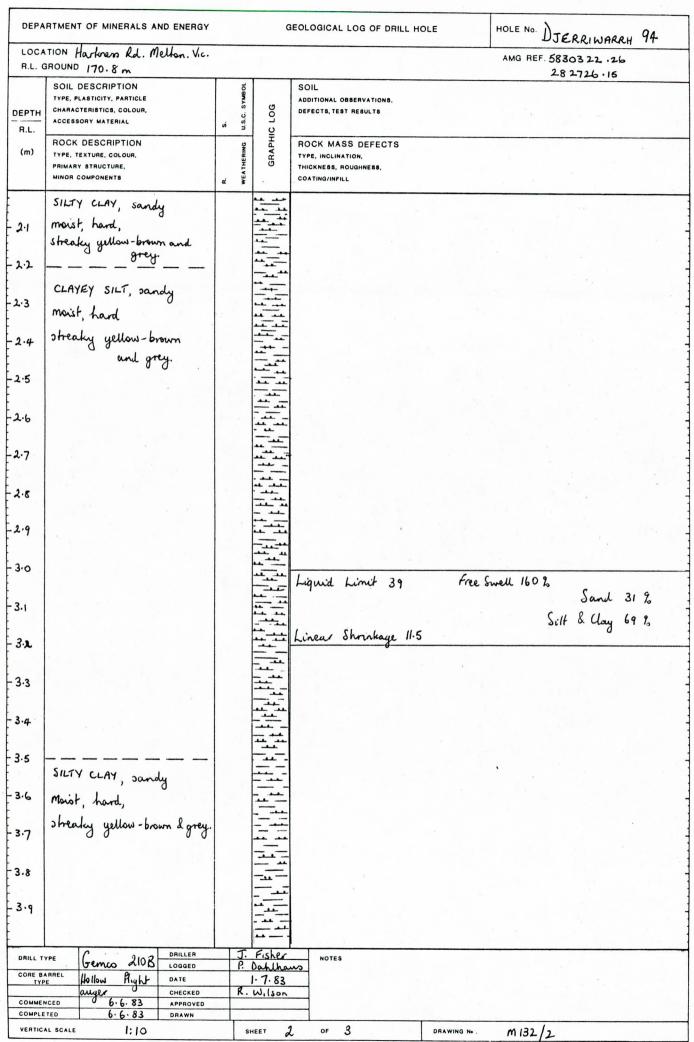
	RTMENT OF MINERALS AND ENERGY		GEOLOGICAL LOG OF DRILL HOLE	HOLE NO. DJERRIWARRH 93
LOCA R.L. G	TION Harkness Rd. Melton. ROUND 166.4 m			AMG REF. 5829801.88
EPTH R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL	s. C LOG	SOIL Additional observations. defects, test results	282651.13
(m)	ROCK DESCRIPTION TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE, MINOR COMPONENTS	R. S. WEATHERING U.S.C. SY WEATHERING U.S.C. SY	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill	
.1	SILTY CLAY, moist, very shiff, mottled yellow-brown and grey. sandy in parts.			
1.2	mottled yellow-brown			
3	and grey.			
4	sandy in parts.			
.5				
6			4 4 4	
.7				
.8				
.9				
.0			5	
5.1				
.2				
.3				
.5				
3.6				•
.7				).
9.8				
3.9				
DRILL TY	JEMIO LOGGED LOGGED	J. Fish P. Duhl	Laus	
COMMEN	E HOLOGY MIGHT DATE OMOGY CHECKED ACED 6.6.83 APPROVED	15.6.8 R.W.14+	<u>n</u>	
COMPLE	TED 6.6.83 DRAWN	SHEET	2 OF 3 DRAWIN	an. M131/2

	RTMENT OF MINERALS AND ENERGY			BEOLOGIC	LOG OF DRILL	HOLE	HOLE NO. DJERRIW	arrh 93
R.L. G	TION Harkness Rd. Melton. Vi ROUND 166.4 m	с.				)	AMG REF. 582980 28265	
DEPTH R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL	ø	HERING U.S.C. SYMBOL GRAPHIC LOG	1	L OBSERVATIONS, EST RESULTS			
(m)	ROCK DESCRIPTION TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE, MINOR COMPONENTS	œ	GRAPH	TYPE, INCL	ROUGHNESS.			
	SILTY CLAY, sandy			Liquid	Limit 65	Free Swel	4 200 % Sand	18 %
4.1	moist, very shiff, mottled yellow-brown & grey.				6, 1		u 200 % Sand Sult & Cla	y 82Z
4.2	0 (O			Linear	Shrinkage 1	4		1
4.3	End of Hole.							
4.4								
4.5								
		1						
4)							•	
					а а			
DRILL T			J. Fishe		FS			
CORE BA	VAREL (1 " D. LOGGED		P. Jahle 15.6.83	ano	1 1 1 1			
TYP	auger U CHECKED		R. Wils	on				
COMMEN	0.0.00							
	AL SCALE 1:10		SHEET	3 OF	3	DRAWING N.	M131/3	

	TION Harkness Rd. Mellon.				AMG REF. 59	330322.26
R.L. G	ROUND 170.8 m					\$2726.15
EPTH R.L.	SOIL DESCRIPTION Type, plasticity, particle Characteristics, colour, accessory material	s. U.S.C. SYMBOL	c rog	SOIL additional observations, defects, test results		
(m)	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components	R. WEATHERING	GRAPHIC	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill	-	
D-1	CLAYEY SILT, dry, blocky, brown.					
0.2	SILTY CLAY, sandy moist, hard, yellow-brown					
	yellow - brown					
0.4						
	calcureous sand layer at 0.6 m					
.7	art 0.6 m					
.9			- + + + + + + + + + + + + + + + + + + +			
.0			1 1 1		6 . (	
.,	SILTY CLAY, sandy			Liquid Limit 48	Free Swell 170 %	Sand 25%
.2	maist, hard, streaky yellow-brown and grey			Linear Shrinkage 14.5		Silt & Uny 75%.
·3	and grey					
4						
•5					•	
·6 ·7						
-8						
.9						
CORE BA	ARREL 11. PL 11 DUTE	P. 1	Fisher Dahlhau 1.7.83 Wilson	NOTES		

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		./		EOLOGICAL LOG OF DRILL HO	LE HOLE NO. DJERRIWARRH 94
R.L. G	TION Harkness Rd. Melton. BROUND 170.8 m	Vic.	1		AMG REF. 5830322.26 282726.15
R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL	s. U.S.C. SYMBOL	GRAPHIC LOG	SOIL additional observations, defects, test results	
(m)	ROCK DESCRIPTION TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE, MINOR COMPONENTS	R. WEATHERING	GRAPH	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill	
4.1	SILTY CLAY, sandy maist, hard, streaky yellow-brown and grey.				
4·3	End of Hole.			Michael Marker (C. 1997) Carlos (C. 1997)	
4.4					
4.5					
•					
					•
. 2					
DRILL T	UPINCO 2100 LOGGED	J	Fisher Dahlha	NOTES	
CORE BA	ARREL Hollow Hight DATE		1.7.83 R. Wilson		
COMMEN	P.0.03 AFFROTED		SHEET 3	OF 3	DRAWING NO. M132/3

	RTMENT OF MINERALS AND ENERGY			GEOLOGICAL LOG C		HOLE NO. DJERRIWARRH 100
LOCA R.L. G	TION Harkness Rd. Melton. V ROUND 170 m. approx.	ic. 1	Adjacent	to Djerriwarri	h Bore 93	AMG REF. 5819807
EPTH R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL ROCK DESCRIPTION	vi	HERING U.S.C. SYMBOL GRAPHIC LOG	SOIL ADDITIONAL OBSERVAT DEFECTS, TEST RESULT	TS	282653
(m)	TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE, MINOR COMPONENTS	æ	GRAPI	TYPE, INCLINATION, THICKNESS, ROUGHNES COATING/INFILL		
D.I				Open S.	tandpipe inst	alled 10.11.183.
).2				Date	Reading	Comment
.3				10.11. '83	7.5 m	Dry
•4	Not sampled.			17. 11. '83	7.5 m	Dry
.5	,			8. 12. '83	7.5m	Dry
.6				29.3.'84	7.5m	Dry
.7				18.7. '84 16.8.'84	7.5m 7.54 m	Dry
.8				19.9. '84	7.59 m	Dry
				17. 10. '84	7.57m	Dry
.9				15 . 11. '84	7.56 m	Dry
	Undisturbed Sample 63mm T.W.T. SANDY GRAVEL, clayey. quarte gravel, subrounded		0.00 0.00 0.00			
.2	SANDY CLAY, moist, very stiff,					
3	moist, very stiff, mottled yellow-brown & grey. Sand is subrounded quartz		· · · · · · · · · · · · · · · · · · ·			
4	quartz					
5	and the second					
6						
7	Not Sampled.					
8						
9						
DRILL TY	Venco 210 D LOGGED		J. Fisher ? Dahlhar	NOTES		
CORE BA	ARREL Solid Hight DATE auger checked		12.'83			
COMPLE	TED         JO-11.33         APPROVED           TED         JO-11.33         DRAWN		SHEET	1 OF 4	DRAWIN	an. M14-7/1

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LOCA R.L. G	TION Harkness Rd. Melton. V., ROUND 170 m. approx.	. Adj	jacent +	o Djerniwarrh Bore 93	AMG REF. 5829807 282653	
DEPTH R.L.	SOIL DESCRIPTION Type, plasticity, particle Characteristics, colour, accessory material	S. U.S.C. SYMBOL	IC LOG	SOIL additional observations, defects, test results		
(m)	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components	R. WEATHERING	GRAPHIC	ROCK MASS DEFECTS TYPE, INCLINATION, THICKNESS, ROUGHNESS, COATING/INFILL		
2.1						
2.2	Not Sampled					
2.3						
2.4						
2.6	Undisturbed Sample 63mm T.W.T. SANDY CLAY,					
2.7	moist, very stiff fissured mottled yellow-brown & grey.					
2.8	Sand sized subrounded quartz lines major					
2.9 3.0	Sand sized subrounded quartz lines major hissure dipping almost vertically through sample.					
3.1						
3.2						
3.3						
3.4 3.5	Not Sampled.					
3.6	r					
3.7						
3.8						
3.9						
DRILL TY	JEMOS dIUD LOGGED	- J.P.	Fisher DahUno	NOTES		
CORE BA	ARREL Solid Flight DATE Ouger CHECKED		12.83.			
COMMEN						

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DEPA	RTMENT OF MINERALS AND ENERGY	<del></del> -		GEOL	DGICAL	LOG OF I	DRILL HO	LE	HOLE No.	ATERO	JARRH 100	
LOCA R.L. C	ATION Harkness Rd, Mellon. BROUND 170 m. approx.	Vic.	Adjace	nt to	Bore	Djerniw	arrh 93			F. 582980		
DEPTH R.L. (m)	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL ROCK DESCRIPTION TYPE, TEXTURE, COLOUR,	vi	WEATHERING U.S.C. SYMBOL GRAPHIC LOG	DEF RO TYP	CK MAS					282653		
	PRIMARY STRUCTURE, MINOR COMPONENTS	œ	WEATH		TING/INFIL	OUGHNE88, L						
- 4.1	Not Sumpled											
- 4.2	Undisturbed Sample 63mm T.W.T.											
t	SANDY CLAY, moist, very shiff, fissured mottled yellow-brown & grey.											
-4·4 -4·5	sand size subrounded quartz lines hissures. some rootlets.											
-4.6												
-4.7	UD. 63mT.W.T. SILTY CLAY, moist, very shiff, mottled yellow brown & grey. indistinct layers.											
-4.9 -5.0				=			•					
-5.1												
- 5.2												
-5.3 -5.4	Not Sampled.											
5.5									•			
- 5.6				2 2 2 3								
-5.8												
-5.9												
	ARAPEL Solid Flight DATE PE Solid Flight DATE CHECKED NCED 10. (1. 83 APPROVED		J. Fish P. Dam 12. '83	hans	NOTES							
VERTIC	ETED 10. 11. 83 DRAWN CAL SCALE 1:10		SHEET	3	OF	4		DRAWING No .	M 147	/3		

DEPA	RTMENT OF MINERALS AND ENERGY		G	EOLOGICAL LOG OF DRILL HOLE	HOLE NO. DJERRIWARRH 100
LOCA R.L. G	TION Harkness Rd. Melton.Vi ROUND 170 m. upprox.	c. Adju	acent h	o Bore Djirriwarrh 93	AMG REF. 5829807 282653
DEPTH R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERIBTICS, COLOUR, ACCESSORY MATERIAL	S. U.S.C. SYMBOL	C LOG	SOIL additional observations, defects, test results	
(m)	ROCK DESCRIPTION TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE, MINOR COMPONENTS	R. WEATHERING	GRAPHIC LOG	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill	
6.1					
6.2					
6.3					
6.4		а а. г.			
6.5					
6.6	Not Sampled				
6.7					
6.8					
6.9					
7.0					
7.1					
7.2	11 1 1 . 8 . 1 /2				
7·3	Undusturbed Sample 63mm T.W.T. SANDY CLAY, mottled yellows-brown & grey. less sand than above sample				
	Black stained BASALT (?) clast at be	ltom.			
7.5	Refusal - BASALT.				
7.6					
7.7					
7.8					
7.9					
DRILL T CORE B TYP	ARREL Bolid Flight DATE augur CHECKED	<u></u> р.	Eisher Dahlh 12. '83	ano	
COMPLE		s	HEET	4 of 4. Drawii	M 147 /4

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COATION Urban Lunch Rutharity Roperty, Harkness Rd, Metter:       AND REF 5830185 34         RL. GROUND TORS m       Samona conservations         International conservations       Samona conservations         RL. GROUND Conservations       Samona conservations         RL. GROUND Conservations       Samona conservations         RL GROUND Conservations       Samona conservations         Mark Property       Samona conservations         Pade brown       Samona conservations         O'3       Model brown         O'3       Model brown         O'4       Julian - brown         O'5       Samoda conservations         O'6       CLAY, sandy         medit, very shiff to hard,       Samona conservations is very fine guarte cond.         Samoda fraction is very fine guarte cond.       Samona fraction is very fine guarte cond.         Samona cond greg.       Samona fraction is very fine guarte cond.         Samona cond greg.       Samona fraction is very fine guarte cond.	rrh 19:	HOLE NO. Djeriwa	EOLOGICAL LOG OF DRILL HOLE			MENT OF MINERALS AND	
BRIL	. 84	AMG REF. 5830198	Harkness Rd, Melton.	roperty,	thority	ON Urban Land A OUND 170.8 m	LOCAT R.L. GR
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	CORE RU RECOVERY, SAMPLE T	20200	ADDITIONAL OBBERVATIONS.	r CO	vi	SOIL DESCRIPTION type, plasticity, particle characteristics, colour,	DEPTH
0.2     pale brown.     San by CLAY, sity     San by CLAY, sity       0.3     moist, vey shift to hard,     Free rood (1 cm gl) at 0.3 m       0.4     yellow-brown     San       0.5     San by CLAY, sandy     San       0.6     San by CLAY, sandy     San       0.7     moist, vey shift to hard,     San       0.8     yellow-brown.     San by CLAY, sandy       0.9     San by CLAY, sandy     San by CLAY, sandy       1.0     San by CLAY, sandy     San by CLAY, sandy       1.1     San by CLAY, sandy     San by CLAY, sandy       1.2     San by CLAY, sandy     San by CLAY, sandy       1.3     yellow-brown.     San by CLAY, sandy       1.4     San by CLAY, moist, veny shift be hard, san     San by CLAY, sandy       1.3     yellow-brown.     San brachin is veny hine quarks cand.       1.4     becomes motHed     San       1.5     yellow-brown and grey.     San       1.6     San     San       1.7     San brachin is veny hine quarks cand.       1.8     Yellow-brown and grey.     San       1.9     San brachin is veny hine quarks cand.     San       1.9     Becomes auburg.     San brachin is veny hine quarks cand.       1.9     San brachin is veny hine quarks cand.			TYPE, INCLINATION, Thickness, Roughness,	WEATHERING GRAPHI	æ	TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE,	(m)
0.2 SANDY CLAY, sulty 0.3 moist, very shift to hard, 0.4 yellow-brown 0.5 CLAY, sandy 0.7 moist, very shift to hard, 0.8 yellow-brown 0.9 1.0 1.0 1.0 1.1 SANDY CLAY, moist, very shift to hard, 1.2 SANDY CLAY, moist, very shift to hard, 1.3 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 yellow-brown 1.4 1.5 1.4 1.5 1.4 1.5 1.4 1.5 1.4 1.5 1.4 1.5 1.4 1.5 1.4 1.5 1.4 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5	<u> </u>	; sub-angular to	quartz growel and sand; sub-rounded.		elly	ary, mable,	
0.3 moist, very shift to hand, 0.4 yellow-brown 0.5 0.6	Soil core						0.2 -
0.5 0.6 CLAY, sandy 0.7 meist, very shift to hard, 0.8 yellow-brown. 0.9 1.0 1.1 SANBY CLAY, maist, very shift to hard, 1.2 SANBY CLAY, maist, very shift to hard, 1.3 yellow-brown. 1.4 becomes mothed 1.5 1.6 1.7 becomes sully. 0.9 1.9 becomes sully. 0.9 1.9 becomes sully. 0.9 0.9 1.9 0.9 0.9 0.9 0.9 0.9 0.9 0.9 0		n	tree root $(1 \text{ cm } p)$ at $0.3 \text{ m}$			roist, very shift to	0.3
06						yellow-brown	0.4
0.6	This walled to						0.5
maist, very shift to hard,       initial initinitial initiali initial initial initiali initial initial	sample 63.5mm					CLAY, sandy	
0.8 gellow-brown. 0.9 1.0 1.1 1.2 SANBY CLAY, moist, very-stiff to hard, 1.3 gellow-brown. 1.4 1.5 gellow-brown and grey. 1.6 1.7 1.6 1.9 becomes subly. 0.10 0.10 1.0 1.1 1.1 1.1 1.2 SANBY CLAY, moisture Content 19.9 % 1.1 1.2 Sand fraction is very fine quarte cand. 1.3 1.4 Becomes motHed 1.5 gellow-brown and grey. 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.8 1.9 Decomes subly. 0.10 0.10 1.9 1.9 1.9 1.9 1.9 1.9 1.9 1.9	Ø				hard,	moist, very stiff to	0.7
1.0 1.1 1.2 SANBY CLAY, moist, very-shift be hard, 1.3 yellow-brown. 1.4 becomes mothed 1.5 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.6 1.7 1.7 1.6 1.7 1.6 1.7 1.7 1.7 1.6 1.7 1.7 1.7 1.7 1.7 1.7 1.7 1.7						jellow-brown.	0.8
1.1     SANDY CLAY,     Moisture Content 19.9 %     In       1.2     maist, very-otiff to hard,     Sand fraction is very fine quarte cand.     Sand       1.3     yellow-brown.     In     Sand fraction is very fine quarte cand.     Sand       1.4     becomes mottled     In     In       1.5     yellow-brown and grey.     In     In       1.6     In     In     In       1.7     In     In     In       1.8     In     In     In       1.9     becomes scilly.     In     In       Instruction     In     In     In       In     In     In     In <t< td=""><td>Soil cor</td><td></td><td>small tree root at 0.9m</td><td></td><td></td><td></td><td></td></t<>	Soil cor		small tree root at 0.9m				
11.2     SANDY CLAY, maist, very-shift behard, gellow-brown.     Moisture Content 19.9 %     The sand fraction is very fine quartz sand.     Interpretender       11.3     gellow-brown.     Interpretender     Interpretender     Interpretender     Interpretender       11.4     becomes mothed     Interpretender     Interpretender     Interpretender     Interpretender       11.4     becomes mothed     Interpretender     Interpretender     Interpretender     Interpretender       11.5     gellow-brown and grey.     Interpretender     Interpretender     Interpretender     Interpretender       11.6     Interpretender     Interpretender     Interpretender     Interpretender     Interpretender       11.7     Interpretender     Interpretender     Interpretender     Interpretender     Interpretender       11.7     Interpretender     Interpretender <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>							
113 yellow-brown. 114 becomes mothled 115 yellow-brown and grey. 116 117 118 119 Denomes sulfy. DBILL TYPE Genco 210 DBILLEA J. Fisher Type Hollow Plight OATE 26.3. 185	Thin walled +	4					1.2
1.4 becomes nottled 1.5 yellow brown and grey. 1.6 1.7 1.8 1.9 Decomes sully. DRILLETYPE Genco 210 DAILLER CORE BARREL Hollow Hight DATE 26.3. 185	sample 63.5n	ne quartz sana.	sand traction is very hir	·	hard,		1.2
1.5 yellow brown and grey. 1.6 1.7 1.8 Decomes sully. DRILLER J. Fisher DRILLETYPE Gence 210 DALLER J. Fisher CORE BARREL Hollow Physht DATE 26.3. '85	Ş					,	
1.7 1.8 1.9 DRILL TYPE Gence 210 CORE BARREL TYPE Hollow Flight DATE 26.3. 185					grey.		
1.7 1.8 1.9 DRILL TYPE Gence 210 CORE BARREL TYPE Hollow Flight DATE 26.3. 185		•				and and an ar	1.6
DRILL TYPE Gence 210 DRILLER J. Fisher Notes CORE BARREL Hollow Flight DATE 26.3.185	Soil core						1.7
DRILL TYPE Gence 210 DRILLER J. Fisher NOTES CORE BARREL Hollow Flight DATE 26.3.'85							1.8
CORE BARREL Hollow Flight DATE 26.3.185	This walled tube 63.5		Moisture Content 17.6%			becomes suby.	
CORE BARREL Hollow Flight DATE 26.3.'85	1	and a subset of spatial states and a state state of the states of the states of the states of the states of the			the state of the second state water state		DRILL TY
			<u>sus</u>				CORE BA
смарт снескер				PCD . J. J	HECKED	anger	
COMMENCED APPROVED COMPLETED DRAWN						ED	

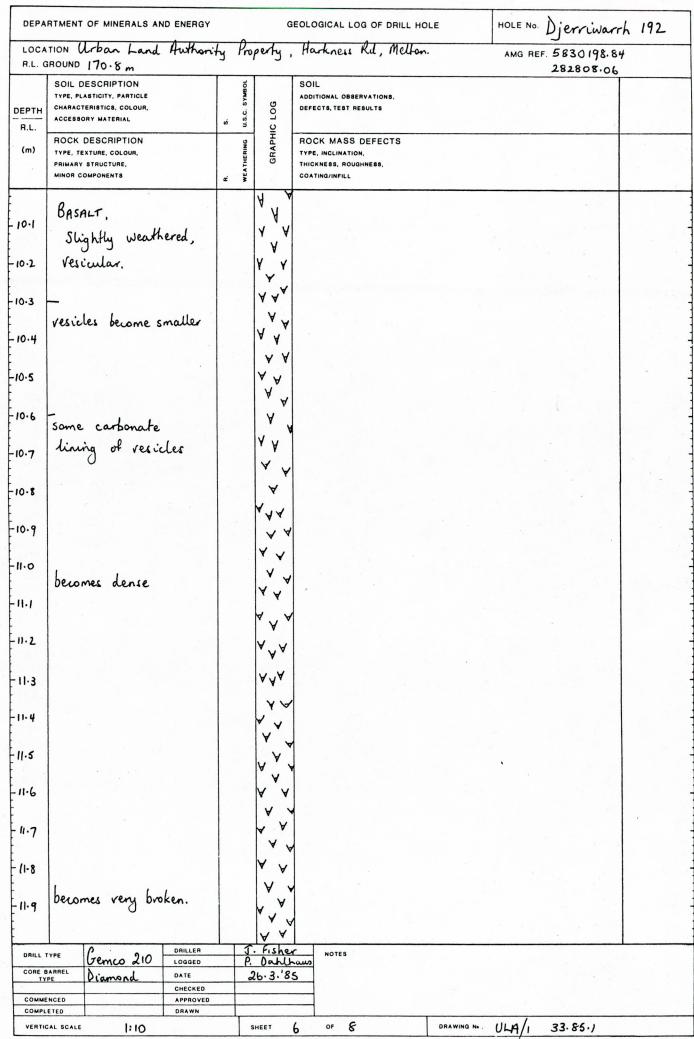
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	A		0	G	Dj	érriwarrh 192
	STION Urban Land Hut BROUND 170.8 m.	honity	Prop	erty		5830198.84 282808·06
	SOIL DESCRIPTION		ğ		SOIL	CORE RUN
	TYPE, PLASTICITY, PARTICLE		SYMBO		ADDITIONAL OBSERVATIONS.	RECOVERY,
DEPTH	CHARACTERISTICS, COLOUR,		U,	LOG	DEFECTS, TEST RESULTS	
R.L.	ACCESSORY MATERIAL	vi	U.S			SAMPLE T
	ROCK DESCRIPTION		g	GRAPHIC	ROCK MASS DEFECTS	
(m)	TYPE, TEXTURE, COLOUR,		R.	RA	TYPE, INCLINATION,	
	PRIMARY STRUCTURE,		WEATHE	0	THICKNESS, ROUGHNESS,	
	MINOR COMPONENTS	œ	×		COATING/INFILL	
	CIAV - 11		L			
2.1	CLAY, sulty.		-			Thin wal
~ /			-		0	tube
in Lenge	moist, very stiff, bio	cley.			Black staining (not unlike dendrites)	sample
2.2	0		ł		, , 0	sample
	maist, very shiff, blo mottled yellow-brown		-		Black staining (not unlike dendrites) along fissured joints.	63.5 m
	and		-		0	ø
2.3	and grey.		-			
	- 0		-			
2.4			F			
,						
			F			Soil
2.5			L			
	2월 - 전문 <sub>2</sub> 6월 - 전문 (1987)		-			Core
			-			
2.6						
			H			
			F			
2.7		·	1		sand filled hissure	
			-			
2.8			L		from 2.75m to 2.9m	· · · · · · · · · · · · · · · · · · ·
×.8			-		none at join to a jim	Soil
			-			
2.9			· -			Core
			-			· · · · · · · · · · · · · · · · · · ·
_			F		mottling becomes very distinctive	
3.0			1			
			1			and the second
3.1			-			ar in the second se
51			-			
			F			
3.2			-			
• •			F			
		10.0	F			
3.3			L.			
			-	++		
			-			
3.4			-			
			-			
2.0			t-			
3.5			-			1
					Very figured undistudies and	Thin
3.6					very fissured undisturbed sample same fine root-hairs down hossures.	walled -
~ 6			-		le le l	
					some line root-hairs down hissures.	sample
3.7			-			
			-		Moisture Content 14.8%	63.5 m
						Ø
3.8				-		
			ł			
•						
3.9			-			
			H			C.1.0
			- l		· · · · · · · · · · · · · · · · · · ·	Soil Cor
DRILL T	TYPE Genco 210 DRIL			Fisher		
CORE B				Dahlh		
TY	PE HOLLOW MIGHT DATE		26.	3.85		an et a a a a
COMME		CKED ROVED				
COMPL						
VERTIC	CAL SCALE 1:10		SHE	ET	2 OF 8 DRAWING NO. ULA/1	33.86.1

DEPARTMENT OF MINERALS AND ENERGY GEOLOGICAL LOG OF DRILL HOLE HOLE No. Djerniwarrh 192 LOCATION Urban Land Authority Property, Harkness Rd, Melton. AMG REF. 5830198.84 R.L. GROUND 170.8 m. 282808.06 SOIL DESCRIPTION SOIL CORE RUN, TYPE, PLASTICITY, PARTICLE ADDITIONAL OBSERVATIONS RECOVERY, CHARACTERISTICS, COLOUR, **GRAPHIC LOG** DEFECTS, TEST RESULTS DEPTH U.S.C. ACCESSORY MATERIAL SAMPLE TYPE. ŝ R.L. ROCK MASS DEFECTS ROCK DESCRIPTION THERING (m) TYPE, TEXTURE, COLOUR, TYPE, INCLINATION PRIMARY STRUCTURE. THICKNESS, ROUGHNESS, WEA' MINOR COMPONENTS COATING/INFILL œ CLAY, sulty. Soil 4.1 core maist, blocky, very stiff. -4.2 mottled yellow-brown Drilling very difficult. Drillers pour water down hole 4.3 and grey. 4.4 Soil so core run is partly wet. Core 4.5 4.6 4.7 4.8 CLAY, silly -4.9 moist, blocky, very stiff - 5.0 grey. Tree noot (3mm Ø) down core -5-1 - 5.2 Soil Core - 5-3 CLAY, sandy. maist, blocky, very stiff - 5.4 some sand along hissures - 5.5 mottled brick-red and grey. - 5.6 5.7 Thin walled Moisture Content 10.8% tube sample 5.8 63.5 mm Ø 1001 GRAVELLY CLAY 5.9 000 Soil moist, crumbly. 6 Core 0.0 J. Fisher P. Dahlhaun . Fisher DRILLER DRILL TYPE NOTES Genco 210 LOGGED CORE BARREL TYPE Hollow Flight DATE 26.3. 85 CHECKED anger COMMENCED APPROVED COMPLETED DRAWN VERTICAL SCALE 3 8 DRAWING No. ULA/1 33.85.1 1:10 SHEET OF

		ND ENERGY				Djerniwa	rt 192
LOCA R.L. G	SROUND 170.8 m	nd Anthor	ity Prope	rty	, Harkness Rd. Melton.	AMG REF. 5830198 282805.0	. 84
EPTH R.L.	SOIL DESCRIPTION Type, plasticity, particle characteristics, colour, accessory material		U.S.C. SYMBOL C. LOG		SOIL additional observations. defects, test results		CORE RUN RECOVERY, SAMPLE TYP
(m)	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components		WEATHERING U		ROCK MASS DEFECTS type, inclination, thickness, roughness, coating/infill		
54	GRAVELLY CLA moist, crumbly		000	-	sub-angular quarte gr		
	mottled brick-red a	und grey	0.0	0 1 0	not much sand-sized mate	rial	Soil
·3 ·4			00	00 0			Core
·5				00			
6	CLAYEY GRAVEL brown-red		000	0 00 00 0			
۰٦ ۲	SILTY CLAY				small root-hairs on hissu	re surfaces	Sail
.9	maist, very stiff, grey	blocky					Core
.0							
•1					Imm & root at 7.1m		
.2				-			
• 4							
5					small root-hairs and roo	s Kilmm Ø	Soil
·6 ·7				_			Core
- 8	becomes mottled brick-red and						
7.9	vick-red and	grey.					
DRILL T		DRILLER	J. Fis		NOTES		
CORE B	BARREL II II PIIL	LOGGED	P. Oah 26.3.		us		
TY	anger	CHECKED	20.3.	00			
COMME		APPROVED					

	RTMENT OF MINERALS AND ENERGY			GEOLOGICAL LOG OF DRILL HOLE HOLE No. Djern	iwarrh 192
LOCA R.L. G	TION Urban Land Authon BROUND 170.8 m	ty I	Property	, Harkness Rd, Melton. AMG REF. 5830 28280	198.84
EPTH	SOIL DESCRIPTION Type, plasticity, particle characteristics, colour, accessory material	<i>v</i> i	U.S.C. SYMBOL	SOIL Additional observations, Defects, test results	
R.L. (m)	ROCK DESCRIPTION type, texture, colour, primary structure, minor components	œ	WEATHERING U GRAPHIC	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill	
	SILTY CLAY		=		Soil wr
3.1	moist, very shiff, blocky			- some rootlets.	
3.2	red-brown and grey mottling.			black staining on fissure surfaces.	
:-3	U			-	
3.4					Soil core
<del>9</del> ·5				_	
8.6					
8.7					
8.8	CLAY, silty. moist, stiff, fissured.			- Highly weathered resicular basalt - around 8.7 m.	
8.9					
9.0				- ] tree root 2 lmm. Ø	Soil core
]•1	BASALT, Highly weathered,		A A -		
1.2	Vesicular BASALT, Slightly weathered		A A .	Diamond Dril	lirig
9.3	vesicular		A A		
9.4	CLAY, grey. some Basalt pieces.				Rock Core
1.5				•	wie
9.6	BASALT, Slightly weathered, Vescicular		AA		
9.7			V	1	
9.8	CLAY, grey.				
9,9	BASALT, Slightly weathered, resicular.		AX		
			J. Fish		
CORE B	The Genco 210 LOGGED		P. Dah	haus	
TYP			26. 3. '	<u>(5</u>	
COMME	NCED APPROVED				
COMPLE	AL SCALE (:10		SHEET	5 OF 8 DRAWING NO. ULA/1 33.1	



DEPTH R.L. (m)	SOIL DESCRIPTION Type, plasticity, particle Characteristics, colour, accessory material	SYMBOL	T		282808.06		
(m)		s. U.S.C. SYA	C LOG	SOIL Additional observations. defects, test results			
	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components	R. WEATHERING	GRAPHIC LOG	ROCK MASS DEFECTS TYPE, INCLINATION. THICKNESS, ROUGHNESS, COATING/INFILL			
	BASALT,		4 4				
	Slightly weathered,		~~ ×				
2.2	Slightly weathered, Very broken.		× ×				
2.3			* *				
12.4			× ×				
12.5	Basalt, Slightly weathered,		<b>A A</b>				
12.6	Vesicular.		~ ~ ~				
12.7			¥ ¥				
12.8	SANDY CLAY,			Core loss attributed to botto	l		R
	grey. to			as drillers report indicates			EC
12.9	mottled red-brown and			as driller's report indicates penetration speed increases w	ith depth.		0 V E
13.0	grey to red-brown.						R
13.1							
13.2	some buckshot grovel. 57% recovery.						0 5
13.3	0						S
13.4							
13.5				Core loss attributed to to	a 'of an		
13.6				on basis of drill penetrati	ion.		Lo
13.7	90 %						S
	core loss						3
13.8							
13.9							
DRILL TY	Genco 210 LOGGED	<u></u> Р.	Fisher			-I	
CORE BA	ARREL Diamond DATE CHECKED		6.3. '85				
COMMEN		_					

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LOCAT R.L. GI	TION Urban Land Auth IROUND 170.8m	onity	P	1	EOLOGICAL LOG OF DRILL HOLE HOLE No. Djerniwan		
		J	.,	operty	, Harkness Rd, Melton. AMG REF. 5830198. 282808.0		
R.L.	SOIL DESCRIPTION Type, plasticity, particle characteristics, colour, accessory material	¢,	U.S.C. SYMBOL	c LOG	SOIL ADDITIONAL OBSERVATIONS. DEFECTS, TEST RESULTS		
(m)	ROCK DESCRIPTION TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE, MINOR COMPONENTS	œ	WEATHERING	GRAPHIC	ROCK MASS DEFECTS TYPE, INCLINATION, THICKNESS, ROUGHNESS, COATING/INFILL	1	
14.1							
14.2					Core loss attributed to top of run		
14.3	90 %				on basis of drillers evidence.		LO
14.4	core loss				(drill penetration speed)		S S
14.5							
14.6							
14.7	- ? - ? - ? -	-					R
	SANDSTONE, cemented. mottled red-brown & white some loose quartz gravel.						RUN CON
14.9	SANDSTONE QUARTZITE						Y Los
	hard, dense, white with red-brown swirls				17% core loss		R
15.1	SANDSTONE,						E C O
-15.3	SANDSTONE, pale yellow-brown, some quartz vecnicig.						VERY
13.4							Y
15.5	83% recovery.						
15.6	End of Hole.	_					<u> </u>
15.7				6		-	
15.8							
- 15 -9							
DRILL TY	YPE Contra 210 DRILLER		J.	Fishe	NOTES		
CORE BA	ARREL N. LOGGED			Dahlh 6.3.'8!			
COMMEN	CHECKED	)					
COMPLE			L		8 OF 8 DRAWING N. ULA/1 33.85.1		

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					BEOLOGICAL LOG OF DRILL HOLE HOLE NO. Djerrin	barrh 193
LOCA R.L. G	TION Urban Land Author ROUND 170 m	rity	Prop	erty,	Harkness Rd, Melton AMG REF. 583015 28284	0.08
DEPTH R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL	vi	U.S.C. SYMBOL	IC LOG	SOIL Additional observations. Defects, test results	CORE RUN RECOVERY, SAMPLE TYP
(m)	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components	æ	WEATHERING	GRAPHIC	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill	
0.1	SILTY CLAY, sandy moist, blocky			3 1                   	grass roots throughout.	
0.2	dark grey.	_			occasional gravel	Soil Core
0.3	CLAY, silly maist, shiff,				very plastic "greasy" clay.	
0.4	grey.			t	some sand and gravel present.	
0.5					U	Thin walled tube
0.6				, 14 1 1 4 1 1 4 1	Maisture content 23.1 %	sample 63.5mm g
0.7					1 cm & root at 0.7m	
0.8	SANDY CLAY,	-				
0.9	monist, very shiff				some culcareous sand around 1 m.	
1-0	brown-grey				very fine quartz sand lining hissures.	Soil
1-1					0	Core
1.2						
1.4						
1.5						
1.6	becomes					
1.7	mottled yellow-brown and grey.				Moisture Content 18.5 %	Thin-
1-8				· · · · · ·		walled fut sample
1-9				· · · · · · · · · · · · · · · · · · ·	Sand lining on hissures.	63.5 mm 5
DRILL T	PE Genco 210 DRILLER LOGGED		J. P.	Fishe		
CORE BA	ARREL UMULAN PI II DATE			9.3.8		
	anger CHECKED	)				
COMMEN						

	RTMENT OF MINERALS AND ENERGY			EOLOGICAL LOG OF DRILL HOLE HOLE No. Djern	iwarrh 193
LOCA R.L. G	TION Urban Land Auth BROUND 170m	onity	Property	, Harkness Rd, Melton. AMG REF. 5830	
DEPTH R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL	S. U.S.C. SYMBOL	C LOG	SOIL ADDITIONAL OBSERVATIONS, DEFECTS, TEST RESULTS	CORE RUN RECOVERY, SAMPLE TYPE
(m)	ROCK DESCRIPTION type, texture, colour, primary structure, minor components	R. WEATHERING	GRAPHIC	ROCK MASS DEFECTS TYPE, INCLINATION, THICKNESS, ROUGHNESS, COATING/INFILL	
2.1	SANDY CLAY, moist, stiff			appears hissured, with open hissures	
2.2	mottled yellow-brown and grey.	1		(but may be just 'discing' on augering) fine quartz sand along hissures.	Soil
2.3	0 0		·····	Imm. & root at 2.5m.	core
2.4		*			
2·5 2·6					
2.7				Maisture content 20.7 %	Thin walled
2.8					tube samp 63.5 mm
2·9 3·0					Ø
3.1	CLAY, sandy	-		small root 2mm ø at 3.7, 3.6m	
3.2	maist, very shift mottled yellow-brown and grey.		· · · · · · · · · · · · · · · · · · ·	Struce 1001 2000 p 01 3.7, 5.6 m	C
	and grey.			not hissured in appearance.	Soil Core
3.4					
3.6					
3.7					
3.8					
3.9			· · · · · · · · · · · · · · · · · · ·		Thin walke tube 63.5
DRILL TY	PE Con 910 DRILLER	J.	Fisher	NOTES	
CORE BA	Genco di Logged	P.	Dahlha	mo	
		29	. 3. 85		
TYP	GALARY V CHECKED				
COMMEN					

	RTMENT OF MINERALS AND ENERGY			BEOLOGICAL LOG OF DRILL HOLE HOLE No. Djeriwo	urh 193
LOCA R.L. G	TION Urban Land Authority BROUND 170 m	y Prope	erly, Ho	arkness Rd, Melton. AMG REF. 5830150. 282841.2	
DEPTH R.L.	SOIL DESCRIPTION Type, plasticity, particle characteristics, colour, accessory material	S. U.S.C. SYMBOL	IC FOG	SOIL ADDITIONAL OBSERVATIONS, DEFECTS, TEST RESULTS	CORE RUN RECOVERY, SAMPLE TYPE
(m)	ROCK DESCRIPTION TYPE, TEXTURE, COLOUR, PRIMARY STRUCTURE, MINOR COMPONENTS	R. WEATHERING	GRAPHIC	ROCK MASS DEFECTS TYPE, INCLINATION, THICKNESS, ROUGHNESS, COATING/INFILL	
4-1	CLAY, sandy moist, very shiff,				τ.ω.i 63mm
4.2	monist, very shiff, mottled yellow-brown and grey.				
4.3	CLAY,			occasional sand grain, some iron (?) staining on hissures.	
4.4	moist, very shift to hard			Some iron (?) staining on hissures. 3mm \$ root at 4.3m	Soil Core
4.5	mottled yellow-brown and grey.				wire
4.6	0.0				
1.7				slickenslide at 4.7m	
1.8					
4.9		i Na Ag			
5.0	CLAY, sandy moist, stiff,				Soil
5.2	mottled yellow-brown and grey.				Core
5.3	und grog			slickenslide at 5.2 m open hissured (discing)	
5.4	becomes silfier and sandier			very fine quartz sand on fissures.	
5.5	SANDY clay,				
5.6	dry to moist, very triable, dessicated,			dessicated crumbly from 5.8 m.	
5.8	mottled brown & grey.		: : 1  : : 1	10 mm. ungular quartz piece at 5.9 m	Sail
5.9	becomes		- -, / #.  -,   #.  -	20 mm 6.0 m	Core.
	SANDY SILT				- /
CORE BA	ARREL Hollow Hight DATE auger CHECKED	J. P. & 9	Fisher Dahlho . 3.'85	NOTES	
COMPLE					

	RTMENT OF MINERALS AND ENERGY				BEOLOGICAL LOG OF DRILL HOLE	HOLE No. Djerriwa	urrh 193
LOCA R.L. G	TION Urban Land Authon BROUND 170 m	ty	Prope	ty,	Harkness Rd, Melton.	AMG REF. 5830150 282841.5	.08
EPTH R.L.	SOIL DESCRIPTION Type, plasticity, particle Characteristics, colour, Accessory material	vi	U.S.C. SYMBOL	C 10G	SOIL additional observations, defects, test results		CORE RUN RECOVERY SAMPLE TYPE
(m)	ROCK DESCRIPTION TYPE, TEXTURE, COLOUR, PRIMARY BTRUCTURE, MINOR COMPONENTS	æ	WEATHERING	GRAPHIC	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, Coating/infill		
o.1	SANDY SILT,		414.4.				
.2	GRAVELLY CLAY,		-	<u>0</u> -	sub-angular quarte gravel;		
	moist, friable		-	0	beds at 6.4 - 6.5 7 parous ( 6.66 - 6.75 graded 6.77 - 6.79 graded	voidy)	
•3	mottled yellow-brown and grey			00	6.77 - 6.79 J gradea	sand beds.	Soil
•4	distinct gravel/sand		1.72.14		6.5-6.66 grey silty	clay.	Core
•5	beds.						
•6			-				
.7				0.73			
-8			11:51	201271 7			
.9		_					Soil
.0	CLAY, sandy		• • •		hissured, desicated		Core
,	moist, hard,						
1.2	moist, hard, mottled yellow-brown and grey.						
.3	0 ()						
.4							Sail Core
.5				· · · · ·	small rooks at 7.5m		lore
1.6			. 1. 1.		Suran incla on 1.3m		
Г-7 				=:-			
.8				 			
7.9			-				Sail Core
	DRILLER	1	J. F	ishe			
CORE BA	Genco 210 LOGGED		P. De	hlha	un		
TYP	E HOLLOW PLIQNE DATE		29.	3. 85			
COMMEN	APPROVED						
COMPLE	TED DRAWN						

	non (labor la Ari		0		BEOLOGICAL LOG OF DRILL HOLE HOLE No. Djerr	warrh 1
LOCA R.L. G	FROUND 170 m	nity	fro	perty,	Harkness Rd. Melton. AMG REF. 58301 28284	
EPTH R.L.	SOIL DESCRIPTION Type, plasticity, particle Characteristics, colour, accessory material	vi	U.S.C. SYMBOL	IC FOG	SOIL ADDITIONAL OBBERVATIONS, DEFECTS, TEST RESULTS	CORE RUN RECOVERY, SOIL TYPE.
(m)	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components	œ	WEATHERING	GRAPHIC	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill	
8.1	CLAY, sandy moist. hard,				dessicated, open hissures, slickenslides.	
3.2	mottled yellow - brown					Soil Core
··3	and grey.					GIE
8·4 3·5					E.W. Basalt piece at 8.4m	
•6					H.W. Vesicular basalt pieces	
•7		1			at 8.6 m to 8.8 m.	Soil Core
.8	 слау,					
.0	moist, very shiff, grey. calcareous nodules				Large calcareous nodule from 8.9-9.0 m.	
.,	throughout					
-2	BASALT, medium dense, large vesicles 1-2 cmp cabonate lined.			A A A A		
1.3	large vesicles 1-2 cm ø cabonate lined.			A . A A . A		
.4				× × × ×		
1.6	CLAY, grey.	-		<u> </u>		
.7	CLAY, grey. 90 % core loss.					
1.9	BASALT			A A A A A		
DRILL T	YPE Commence 910 DRILLER			Fisher	NOTES	
CORE BA	ARREL Hollow flight DATE auger CHECKED		P. 29	Dahlh. . 3. '85	aus_	

	RTMENT OF MINERALS AND E			GEOLOGICAL LOG OF DRILL HOLE	HOLE No. Djern	warrth 193
LOCA R.L. G	BROUND 170 m	Authority	y Propert	y, Harkness Rd, Melton.	AMG REF. 5830/50 282841	
R.L.	SOIL DESCRIPTION TYPE, PLASTICITY, PARTICLE CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL	ø	U.S.C. SYMBOL	SOIL additional observations, defects, test results		CORE RUN, RECOVERY, SAMPLE TYP
(m)	ROCK DESCRIPTION Type, texture, colour, primary structure, minor components	œ	GRAPHIC U.GRAPHIC	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, coating/infill		
10-1	BASALT,		A A	Rf= 2		kock core
10.2	medium dense, some alteration.		V V	,		
0-3			A V	, Rf ≈ 4		
10.4				/		
10.5						
0.6			V V	, Rf = 4		0.
0.8			V V	/		Kock core
0.g				Rfe 6		
1.0			V V V			
11-1			V V	/		
11.2			V V	, RF- 10		
1.4			V V			
1.5			A A			
1.6			A A	R#= 6		
11.7				/		
11.8			V V	Rf=6		
11.9			V V V V	,		
DRILL T		ILLER	J. Fish P. Dahly		s per 0.3m	
CORE BA	ARREL Diamond DA	TE	29.3.18		. p	
COMMEN	NCED AP	ECKED				
COMPLE	AL SCALE 1:10	AWN	SHEET	6 OF 8 DRAWI	NG N. ULA/2 33.85.	

LOCA	TION Urban Land			BEOLOGICAL LOG OF DRILL HOLE Harkness Rd. Meltan.	HOLE No. Djerrin	
	ROUND 170m			in the second second	AMG REF. 5830150 282841	
EPTH R.L.	SOIL DESCRIPTION Type, plasticity, particle characteristics, colour, accessory material	ά	U.S.C. SYMBOL	SOIL additional observations, defects, test results		CORE RUN RECOVERY, SAMPLE TYP
(m)	ROCK DESCRIPTION type, texture, colour, primary structure, minor components	œ	GRAPHIC U.	ROCK MASS DEFECTS Type, inclination, Thickness, roughness, Coating/infill		
2.1	BASALT, medium dense, some alteration.			Rf = 6		
2.2	medium dence, some alteration.			NF = 10		Rock core.
2.3						
2.4				Rf = 12		0
2.5			V V			Rock core
2.6			V V			
2.7			V V	Rf. 20		
2.8			A A A			
3.0			v v			
3.1			V	Rf. 20		
3.2				(		
3.3			V V			
3.4	GRAVELLY CLAY grey, becomes	۶,	000	27 % core recovery.		Rouk
3.5	grey, becomes red-brown.					very k
3.6	some buckshot g		0-			c c
3.7	27% recover	eng.				L e
3.8						s S
3.9						
DRILL TY	YPE Genco 210	DRILLER	J. Fishe P. Dahlh	NOTES		
CORE BA	ARREL A.	DATE	29.3.18		0.3 m	
		CHECKED				
COMMEN	the state of the s	APPROVED DRAWN				

I

		T OF MINERALS A	ND ENERGY		GI	EOLOGICAL LOG OF DRILL HOLE	HOLE No. Djerriwa	urch	193
LOCA R.L. G		Urban Lan 170m	d Autho	rity	Property	y, Harkness Rd, Melton.	AMG REF. 5830150 2828411	0.08	
	SOIL	DESCRIPTION		SYMBOL		SOIL ADDITIONAL OBSERVATIONS,	2020 71.	06	
R.L.	CHARAC	CTERISTICS, COLOUR, SORY MATERIAL		S. U.S.C. SY	DO1 C	DEFECTS, TEST RESULTS			
(m)	TYPE, T	DESCRIPTION TEXTURE, COLOUR, Y STRUCTURE,		R. WEATHERING	GRAPHIC	ROCK MASS DEFECTS Type, inglination, Thigkness, roughness,			
		COMPONENTS		R. WEAT		COATING/INFILL			
14.)								l	Ro
4.2								S	lor
4.3								S	
14.4							•		
4.5	SII-	гу сћау,							
4.6	Hed,	-brown wi	th						0
7.0	occ	iossional							R
4.7	g	-brown mi cossional rewel piece	L.						Con
4 ·8	bec	omes							-4
4.9	SAN	omes VDY CLAY, Hed while a							
5.0	mot	Hed while a	and						
5.1	red	- brown							
15.2		? - ` -?	· · ·?.						
5.3	CEN	IENTED SAN	ids,		· T.				
5.4	white	he with red uks. 1e guartzos	- brown		J				
5.5	Sem	le quartzos	e gravel.						
15.6					ST				
15.7					:l: -::				
5.8	3A yel	NDSTONE, Low-brown							
15 9				. **					
	End	of Hole.							
DRILL T	YPE	Genio 210	DRILLER	- J Р.	. Fisher Dahlha	NOTES			
CORE BA		Diamond	DATE		9.3.'85				
COMMEN	NCED		CHECKED						
COMPLE			DRAWN						

APPENDIX II EXCAVATION LOGS

DE	PART	MENT	OF MINERALS AND ENE	RGY			ON LOG.		PIT	No. ]	JER	RIWAR	RH	175
PF	OJEC	T Prop	osed Cemetery at	Melton Via	LOC R.L.	GROL	ind ilg	.88			AMO	3. REF	30	010. 79.3
EQ	UIPM	ENT T	YPE AND MODEL		m LOP				а на 1999 годи на Кончира такон				<u> </u>	1.1-2
		-	SOIL DESCRIPTION		ă LON		m v	VIDE					s	HAND
	2		TYPE, PLASTICITY, PARTICLE		3	ø								ENETRO-
	EASE OF EXCAVATION	2	CHARACTERISTICS, COLOUR, ACCESSORY MATERIAL		soil	LOG							-	
	EASE OF	Ξ Ε				Q		******						STRENGT
ME I HOD	E A	DEPTH	ROCK DESCRIPTION		1	GRAPHIC			*					
H	AS	L L	PRIMARY STRUCTURE		HOCK	AA						ATE	< "°	INT LOAD
-			WINOR COMPONENTS			Ø						3		
			CLAY, silly				Materi	ial in a	Inbside	ed p	ortion			
		0.2 -	maist, hiable, shiff,	hissing										
			i indust, madre, storr,	rissurea			CLAY,	slightly	sandy					
		0.4	brown. many tree roots.					0 0	. 0					
			many thee rook.				uny to	moist	blocky.					
		0.1		transferrar provinces have been			brown.		0					
		0.6	SILT, chuyey fine coloureous material		1						,			
			Fine calcureo us muterial	throughout.			open j	ointed,	some 2	0 mm	wide	,		
		0.8	CLAY CLIL				many +	hee roots	•					
			CLAY, slightly sandy.				0							
		1.0	- moist, stiff.											
			yellow brown.						p			-		
		1.2					CLAY, sl moist, shi textured.	ightly 3	andy. to	ssure	ld.			
			becomes mottled yells	ow-brown			textured	IF, yello	in to so	ft or	1 oper			
		1.4	and grey around 1.					· dome !			a.s			
		1.6	-											
		1.8					-							
			-1		I	1	I							
	-		1.0 2,	0	3.0	3.4								
0	$_{2}$	Sur P		- A							-			
0.			14. 3 C	and the state of the state		< - c	LAY, Silly	,						
			Darte Com in a Long Art State	and the second		1	9							
0.			The second second	19		-51	T							
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,	•4			And the second second	14				p	+	-		+-1	
۱	.6					- 64	ly, slight	y sandy	, fissur	ed.				
1	1-8-0	LHY sh	X Want and and the second											
-	2-o-b	locky,	open joints											
										1				
•														
	•													
Lo	gged		PDallhaus	Notes										
	gged ate		P. Dahlhaus	Notes										
D		d	P. Dahlhaus 10. 5. '84	Notes										
D	ate		The second s	Notes										

DEPARTMENT	OF MINERALS AND ENERGY	EXCAVAT	ON LOG.	PIT NO. DJERRIN	SARR	н 176
	oposed Cemetery at Me	LOCATION Iton Vic.R.L. GROU	I IND 169.88	AMG. F	REF.	0010.7 879.3
	TYPE AND MODEL	m LONG	m WIDE			
TION	SOIL DESCRIPTION Type, plasticity, particle characteristics, colour, accessory material	soil. USC SYMBOL				S HAND PENETRO- METER LP
METHOD EASE OF EXCAVATION	ROCK DESCRIPTION TYPE TEXTURE, COLOUR, PRIMARY STRUCTURE WINOR COMPONENTS	FOCK WCATHERING			WATER	P STRENGT
H 0.0	CLAY, silty, moist, stiff, greenish - yellow brown. Slightly sandy.		CLAY, silly. moist (near dry brown.	i) very stiff, blocky. I) very stiff, blocky. Ins patches(sandy)		
2.0 Logged	P. Dahlhaus		17, silky , blacky ,	epen-jointed		
Date Checked Approved	10.5.84					
Vertical Sci	ale 1:20	Sheet no. / o	f / Dra	wing no. Subs. No	2	

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DEPA	RTN	MENT	OF	MI	NER	ALS	S A	ND	EN	ER	GΥ			E	xc	VA	TIO	NL	OG				P	IT I	No.	D2	TER	RIW	ARR	н Г	77
PRO		TPre	pose	ed	Ce	eme	ter	4_	at	M	elt	on	Vic	L . Fi	OC/	GRO	)N DUN	D ] (	69	• 4	8									178. 828	
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	NO			CHA	E. PLAS RACTE	RISTIC	s. co	LOUR						SOIL	USC SYMBO	LOG.														WETE	
	/ATI	ε			ESSOF									5		0	+													H STA	LL.
EASE OF	EXCAVATION	DEPTH		TYPE	CK D TEX	URE.	coror							ROCK	ME A THE RING	BRAPHIC													WATER	POINT	
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		0.8	F	CLA	Y, s	illy,																									
				mois	it, s	hiff																									
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DEPARTMENT OF MINERALS AND ENERGY	EXCAVATION LOG. PIT NO. DJERRIN	NARRH 178
PROJECT Proposed Cemetery at Me	LOCATION	5829972.( REF. 282839.3
EQUIPMENT TYPE AND MODEL EXCAVATION DIMENSIONS	m LONG m WIDE	202057.3
DIL E	soit. LOG.	S HAND PENETRO- METER 194
NOILE     E       COULE     CHARACTERISTICS. COLOUR.       ACCESSORY MATERIAL       COULE       COULE	MOLK WOLK MINIMUL	POINT LOAD
B CLAYEY SILT, Sandy. fissur moist friable dorkgrey. DEP CLAY, silty. CLAY, silty. CLAY, silty. CLAY, silty. CLAY, silty. CLAY, silty. To moist, shift, fissured greenish - brown. Sightly sandy. 2.0 SANDY CLAY, moist, very stiff, 2.5 moithed brick-red and grey. 3.0	Some mottled patches. Sand pockets (fine quarts sand) throughout. One large sand-filled fissure (60 mm to 2mm fissure wridth)	
	3.0 4.0 5.0 6 SILT Sponday	
1.4 1.6 1.8 2.0 2.2 2.4 2.6 Logged P. Dahlhaus	Notes	
Date IO · 5 · '84 Checked Approved		
Vertical Scale J: 50	Sheet no. 1 of 1 Drawing no. Subs. No. 4	4

		TM	ENT	OF	MIN	ER	ALS	A	ND	EN	ER	GΥ				EXC	CAV	AT	ION	LO	G.				PI	IN	••	DI	ER	RIN	IAR	R F	11	7
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APPENDIX III GEOPHYSICAL REPORT

# EXTRACT FROM UNPUBLISHED REPORT 1985/59 BY W L KILLEY

# METHOD AND EQUIPMENT 1 Equipment

A Geometrics 24 channel signal enhancement seismograph model ES-2415F was used to record the signals from an array of 8 hertz Geospace GSC-11D geophones.

Two seismic energy sources were tested in the area including a Betsy Seisgun surface source and Anzite explosives fired in shallow boreholes. The surface source performed poorly because of its low energy and the high attenuation of seismic energy found at this site. Explosives produced a more energetic impulse and were used for most of the survey.

The explosives were electrically fired and the seismograph triggered by an Electro-Tech BC8A blaster box.

# 2 Field Method

The seismic refraction method was employed in this investigation. A 480m east-west traverse consisting of 7 spreads, and a 620m north-south traverse consisting of 9 spreads were surveyed (see Figure 3A).

Each spread was 69m long with a geophone spacing of 3m.

Five explosive shots were fired for each spread; a centre shot located between geophones 12 and 13, two off end shots offset by 1m and two long shots offset from the ends of the spread by 70m. The sizes of charge used at the centre, off-end and longshots were 150, 250 and 500 grams respectively.

Shot holes of about 1m depth were drilled by a Minute Man motorised auger.

3 Interpretation Method and Record Quality

The Hawkins' (1961) intercept time method was used to compute depths to refractors at shot point locations. Minimum depths were calculated where usable data was sparse.

Record quality was generally fair to good.

Some of the data was irregular in nature and could not be computed by the Hawkin's (1961) technique. This data was attributable to irregular interfaces between refractors combined with the shallowness of the depth of investigation (Redpath, 1973).

# RESULTS

1 East-west Traverse

A surficial layer exists along the traverse and is characterised by seismic velocities 200 to 430 m/sec. At the western part of this profile, the layer varies in thickness from 7.1m at chainage 35m to 2.6m at chainage 310m, and ranges in velocity from 330 to 430 m/sec. This layer thins to less than 0.9m at chainages 380 to 520m, and has predominantly lower velocities (200 to 260 m/sec). The layer thickens to 4m over chainages 550 to 580m and the velocity increases to 390 m/sec. This layer is interpreted to consist of unconsolidated clays, silts and sands.

The surficial layer overlies a layer which has seismic velocities in the 500 to 1880 m/sec range. Lateral velocity changes are apparent.

Two velocity zones exist within this layer :

- a low velocity zone (500 to 760 m/sec) between chainages 375 and 570m, with a maximum thickness of 9.4m at chainage 450m, and
- 2) a high velocity zone (1020 to 1880 m/sec) at chainages 35 to 240m, 275 to 415m and 520 to 575m, which has a maximum thickness of 6.8m at chainage 375m.

The low velocity zone overlies the high velocity zone at chainages 375 to 415m and 520 to 575m. An anomalous velocity of 840 m/sec was detected at chainages 335 to 370m.

Drillhole information from Djerriwarrh 159, 161 and 192 was used to interpret this layer to consist of clay, silt and highly weathered basalt. The lack of borehole data in the area prevented further subdivision of lithology within this layer.

The deepest refractor detected has seismic velocities between 1960 and 3090 m/sec. This refractor directly underlies the surficial layer at chainages 240 to 275m and 575 to 600m, and was interpreted to consist of weathered to fresh basalt. Depths to this layer vary from 3.6m at chainage 275m, to 9.8m at chainage 450m. Fresher basalt interpreted to have the highest velocities (2600 to 3090 m/sec), was detected at chainages 35, 370, 410 and 550m. Depths could not be calculated because of the lack of usable data.

2 North-south traverse

A surficial layer characterised by seismic velocities 240 to 480 m/sec exists along the profile. It ranges in thickness from 2.0m at chainage 105m to 4.4m at chainage 415m. Lateral velocity variations occur in the layer. Data from boreholes Djerriwarrh 161, 159 and 192 at chainages 8, 235 and 475m respectively correlated with this layer which is interpreted to consist of unconsolidated clays and silts and sands.

The surficial layer directly overlies a layer characterised by velocities in the 880 to 1570 m/sec range at chainages 45 to 170m, 210 to 335m and 425 to 450m. This underlying layer has a maximum thickness of 7.9m at chainage 275m and exhibits lateral variations in velocity along the profile. The layer is interpreted to consist of clay, silt and weathered basalt.

For the rest of the traverse, the surficial layer directly overlies the deepest refractor which is characterised by velocities between 2330 and 2900 m/sec. The depth to this layer varies from a minimum of 2.8m at chainage 380m, to a maximum of 10.2m at chainage 275m, and is interpreted to consist of weathered to fresh basalt. The interpreted depth to this layer agrees with data from borehole Djerriwarrh 159.

Lateral velocity variations occur along all layers in both profiles. This could be due to changes in weathering, lithology or a combination of these two factors. However, specific causes are not known because of insufficient borehole information. Further drilling has been recommended (see Fig. 3B).

# 3 Accuracy of Interpretation

Analysis of the uncertainties involved in the calculation of refractor depths was not possible because of the lack of coincident borehole and seismic refraction data.

The Hawkins' (1961) method used to interpret this data assumes planar, shallow dipping layers. The refractor interfaces in this area are irregular and are therefore difficult to accurately delineate with Hawkins' (1961) technique. Consequently further drilling is recommended to confirm the interpretations presented in this report. Refractor depths could not be calculated at Djerriwarrh 161 and 192. However the following observations were made :

- the two interpreted profiles show an increase in depth to basalt towards Djerriwarrh 192, which is supported by the borehole data, and
- 2) borehole data from Djerriwarrh 161 on the north-south traverse shows a surficial layer which is 3.1m thick and directly overlies basalt. This agrees with the trend of the interpretation near this borehole.

The two traverses intersect at borehole Djerriwarrh 192 (see Figure 3B). Agreement of the two profiles at the intersection point is difficult to determine because of the sparseness of the data in this area. A general deepening of the basalt on both traverses was observed. However the velocities detected on each traverse could not be correlated.

# 5 CONCLUSIONS

1) The Hawkins' (1961) interpretation method was not strictly applicable to this data because of the irregular nature and shallowness of the refractor interfaces. Consequently, more borehole information is required to confirm the interpretation.

2) The thickness of material overlying the higher velocity basalt ranges from 3.6 to 9.8m along the east-west traverse, and from 2.0 to 10.2m along the north-south traverse.

# 6 RECOMMENDATIONS

Four boreholes drilled to basalt are recommended (see Figure 3B) to confirm the interpretations presented in this report.

East-west traverse : Two boreholes are recommended for drilling at chainages 35 and 450m. Data from these holes will;

- reveal the precise nature and extent of refractors which have velocities that could not be correlated absolutely with existing borehole data (i.e. 1880 and 760 m/sec at chainages 35 and 450m respectively),
  - 2) confirm refractor depths where minimum depths were calculated, and
  - 3) confirm refractor depths calculated where seismic refraction data coverage was regular and complete (i.e. at chainage 450m).

North-south traverse : two boreholes are recommended for drilling at chainages 105 and 310m. Data from these holes would;

- identify the nature and thickness of the refractors with velocities 1260 and 920 m/sec, and
- 2) confirm interpreted refractor depths where data coverage was good (i.e. chainage 310m).

# REFERENCES

- Dahlhaus, P G, 1985 : An engineering geological investigation of the Urban Land Authority's property, Melton. <u>Department of</u> <u>Industry, Technology and Resources, Geological Survey</u> Division, Unpublished Report 1985/57.
- Hawkins, L V, 1961 : The reciprocal method of routine shallow seismic refraction investigations. Geophysics 26(6), 806-19.
- Redpath, B B, 1973 : Seismic refraction exploration for engineering site investigations. <u>Department of Lands, Surveys and</u> <u>Mining, Geological Survey of Papua New Guinea, Technical</u> <u>Report E-73-4</u>.

APPENDIX IV SOIL REPLACEMENT METHOD (FROM CHEN 1975)

## SOIL REPLACEMENT

A simple and easy solution for slabs and footings founded on expansive soils is to replace the foundation soil with nonswelling soils. Experience indicates that if the subsoil consists of more than about 5 feet of granular soils (SC-SP), underlain by highly expansive soils, there is no danger of foundation movement when the structure is placed on the granular soils. The mechanics and the path of surface water seeping through the upper granular soils and into the expansive soils is not clear. It is concluded that either seepage water has never reached the expansive soils, or the heaving of the lower expansive soils is so uniform that structural movement is not noticeable.

This is not true in the case of man-made fill. For economic reasons, the extent of the selected fill must be limited to a maximum of 10 feet beyond the building line. Therefore, the possibility of edge wetting exists. A guideline has not been established as to the thickness requirement for the selected fill. A minimum of 3 feet should always be insisted upon, although 5 feet is preferred. This thickness refers to thickness of selected fill beneath the bottom of the footings or bottom of floor slabs.

The pertinent requirements concerning soil replacement are the type of replacement material, the depth of replacement, and the extent of replacement.

#### Type of material

Obviously, the first requirement for the replacement soil is that it be nonexpansive. All granular soils ranging from GW to SC in the Unified Soil Classification System may fulfill the nonexpansive soil requirement. However, for clean, granular soils such as GW and SP, surface water can travel freely through the soil and cause wetting of the lower swelling soils. In the other extreme, SC material with a high percentage of plastic clay sometimes will exhibit swelling potential. The following criteria have been used with a certain degree of success:

Liquid limit, percent	Percent minus No. 200 sieve	
Greater than 50 $30 - 50$	15 - 30 10 - 40	
Less than 30	5 - 50	

It is becoming increasingly difficult to locate materials, fulfilling the above requirements, in expansive soil areas such as Metropolitan Denver. If necessary, the requirement for imperviousness can be forfeited. Any selected fill will be satisfactory provided the material is nonexpansive. Also, swell tests are the only positive method of determining the expansiveness of the material. When in doubt, such tests should be conducted rather than relying on plasticity tests.

A great deal of emphasis has been given to the possibility of blending granular soil with the on-site swelling soils, thus reducing the amount of imported fill required. Theoretically, such a method is reasonable; but in practice it is difficult to incorporate granular soil with stiff, dry expansive clays. Disc harrows and plows will be required to break the clay into reasonably sized clods. Such an undertaking will probably be as expensive as using the lime stabilization method.

#### Depth of replacement

The depth of influence is a most complicated question that must be answered when dealing with soil treatment beneath the slabs or footings. To what depth should the natural soil be recompacted? How many feet of overexcavation will be required? How many cubic yards of nonexpansive soil will have to be imported? These questions cannot be intelligently answered until the amount of movement that will occur beneath the slabs or footings can be assessed.

Theoretically, the amount of uplift can be evaluated from the data derived from swell tests and pressure distribution methods. Gizienski and Lee [70] evaluated the theoretically computed uplift derived from laboratory test data and the actual measurement taken from a small scale field test. They found that the actual heave in the field was only one-third of that estimated from the results of laboratory tests.

The Colorado Highway Department established curves which show the relationship between total swell and the depth below the surface of the subgrade [71]. Studies have shown that the swelling can take place down to a depth of as much as 50 feet. Also, 60 percent of the swell in many of the Colorado subgrade clays can occur down to a 20-foot depth.

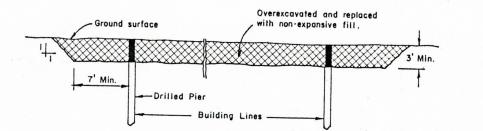
While both the theoretical approach and actual measurement concerning depth of influence are urgently needed, the following should be pointed out:

- 1. The potential vertical rise of a soil mass, say 10-by 10-by 3-feet, (such as that used in Gizienski's experiment) under uniform saturation conditions, can be less than that of the same mass subject to local wetting only. Uniform wetting tends to equalize heaving.
- 2. There is a definite gain in placing the structure on a nonexpansive soil cushion. Even if the deep seated soils swell, the movement will be more uniform, and consequently, more tolerable.
- 3. The depth of selected fill should never be less than 36 inches and preferably 48 inches. The swelling potential of the soil beneath the fill is very important as density and moisture conditions change at various locations. It should be noted that with 4 feet of fill plus the weight of concrete, a uniform pressure of about 600 psf is applied to the surface of expansive soils. For moderately swelling soil, such surcharge load can be important in preventing potential heave.
- 4. The failure of the soil replacement method generally occurs during construction. If the subgrade or open excavation becomes wetted excessively before the placement of the fill, the trapped water will cause heaving. In such case, detrimental heaving will occur regardless of thickness of the selected fill. The soils engineer should have the opportunity of supervising the placement of fill, or such a scheme should not be adopted.
- 5. The thickness of the imported fill can be reduced if a combination of the soil recompaction and soil replacement methods is used. The natural soil is scarified and recompacted as described under "Compaction Control" for a thickness of about 2 feet, then another 2 feet of selected compacted fill placed. The combined thickness of 4 feet should be adequate to control heaving.
- 6. The degree of compaction of the selected fill depends upon the type of supporting structure. For supporting slabs, 90 percent of standard Proctor density should be adequate. For supporting footings, a degree of compaction of 95 to 100 percent should be achieved.

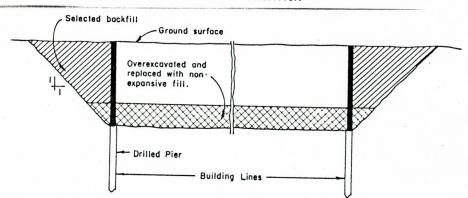
### Extent of replacement

The main reason that an artificially selected fill cushion is less effective than a natural granular soil blanket is that in natural conditions, the blanket extends over a large area, much larger than in the artificial condition. In an artificial fill situation, it is always possible for surface water to seep into the deep-seated expansive soil at the perimeter of the fill. Therefore, the larger the area of replacement, the more effective the fill.

Figure 108 shows the suggested extent of replacement for both basement and nonbasement conditions. With this arrangement, the possibility of surface water entering the foundation soil is greatly reduced. The type of material used for backfill should be the same as used for the underslab selected fill.



## NON · BASEMENT CONDITION



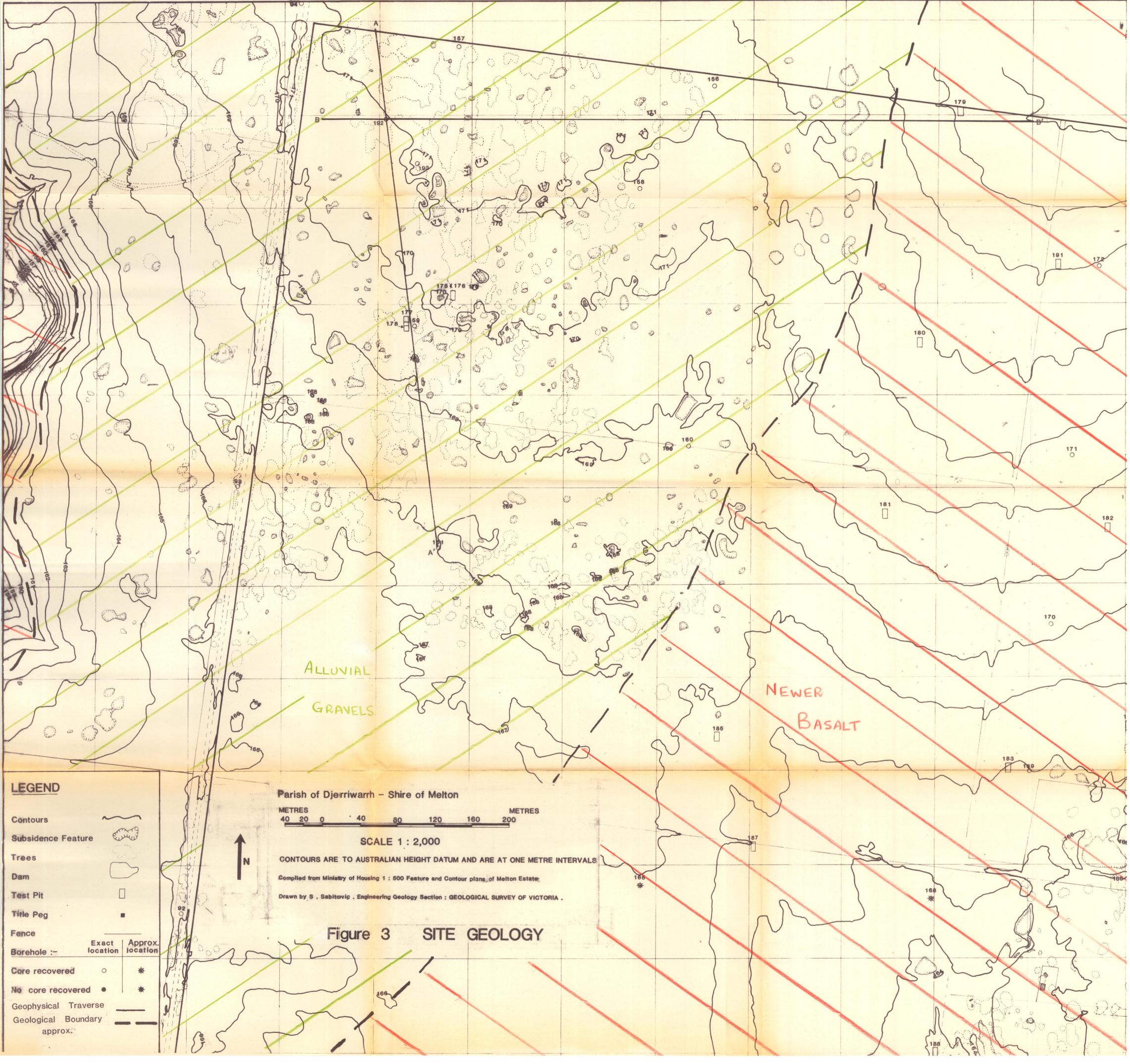
# DEEP BASEMENT CONDITION

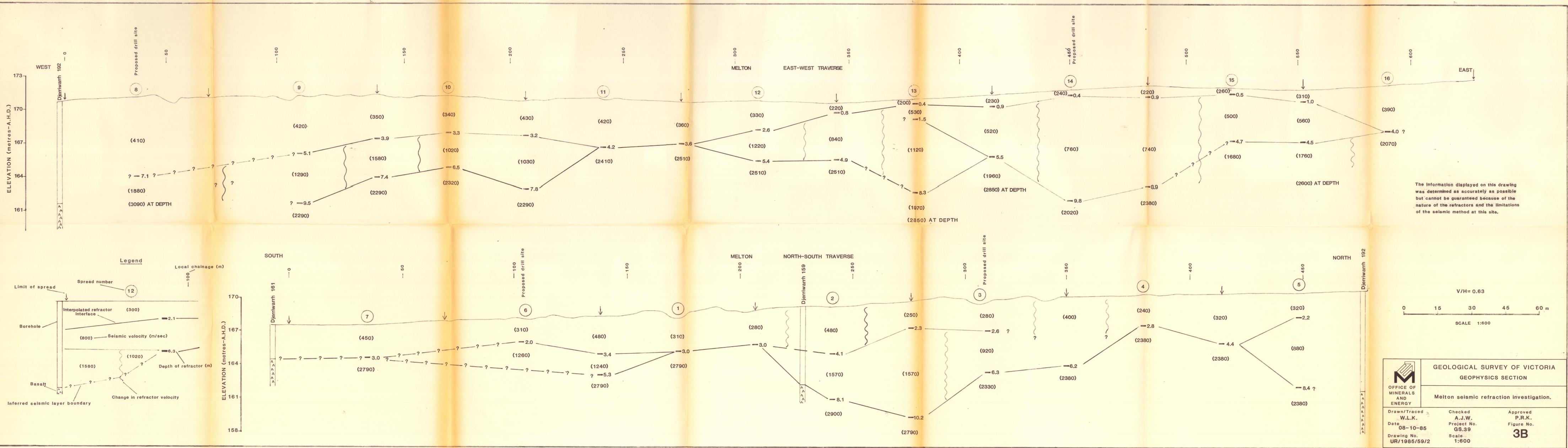
Figure 108. Suggested extent of fill replacement.

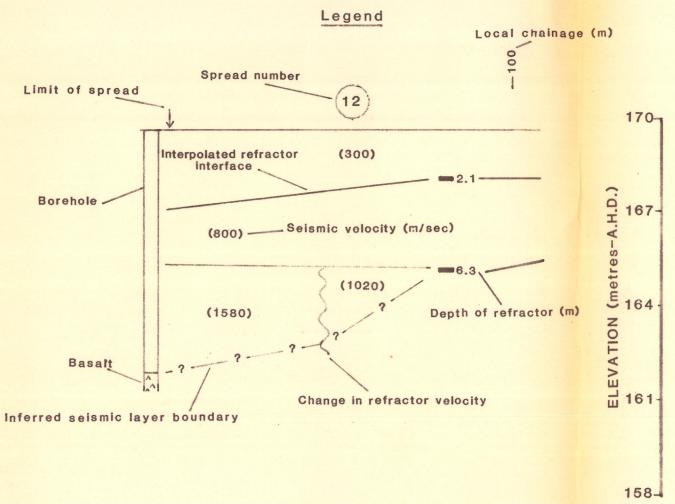
### Evaluation

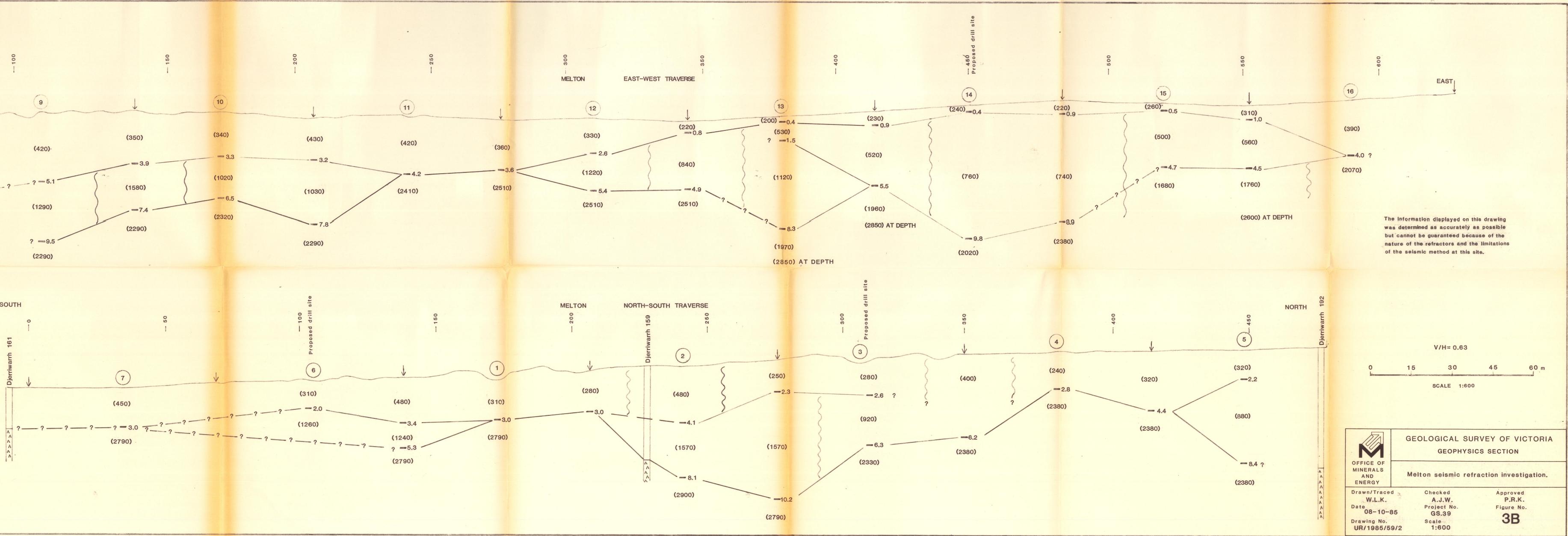
With present technology on expansive soils, soil replacement is the best method to use in obtaining a stabilized foundation soil. The following are the evaluations of soil replacement method:

- 1. It is possible to compact the replaced nonexpansive soil to a high degree of compaction, thus enabling the material to support either heavily loaded slabs or footings. Such capability cannot be obtained by the prewetting method. Also, with the compaction control method, a high degree of compaction on expansive soils is not desirable, and, consequently, the load carrying capacity is limited.
- 2. The cost of soil replacement is relatively inexpensive when compared to chemically treating the soil. No special construction equipment, such as disc harrow, spreader, or mixer will be required. The construction can be carried out without delay as is encountered in the prewetting method.
- 3. The granular soil cushion also serves as an effective barrier against the rise of ground water or perched water.
- 4. With the exception of a structural floor slab (suspended floor), soil replacement provides the safest approach to slab-on-ground construction.
- 5. To guard against unexpected conditions which might cause heaving, it is strongly suggested that floating slab construction be used. Slip joints must be provided for all slab-bearing partition walls so there is no chance of slab movement disturbing the structure.
- 6. Surface drainage around the building must be properly maintained so there is no opportunity for water to enter the expansive soils beneath the selected fill.









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